











CONTENTS

LIST OF TABLES	
LIST OF FIGURES	IV
INTRODUCTION	1
1.1 Background	2
1.2 Survey Objectives	2
1.3 Organisation of Report	2
SURVEY METHODOLOGY	3
2.1 Survey Introduction	4
2.2 Traffic Surveys	4
2.3 Parking Surveys	11
2.4 Origin-Destination Data Survey	15
TRAFFIC SURVEYS	17
3.1 Survey Tasks	18
3.2 Survey Results	18
3.3 Survey Observations	51
3.4 Queue Length Survey	51
PARKING SURVEYS	65
4.1 Survey Tasks	66
4.2 Survey RESULTS	66
ORIGIN-DESTINATION	85
DATA SURVEY	85
5.1 Survey Zoning System	86
5.2 Origin-Destination Survey UniT Matrix	88
5.3 Origin-Destination Survey Result Matrix	89
SURVEY DATA UTILISATION	91
6.1 Data Utilization Introduction	92
6.2 data conversion process	92

NEXT STEPS	97	
7.1 Next model stages	98	
7.2 Next deliverable stages	98	
APPENDICES	99	
APPENDIX 1: SURVEY OBSERVATIONS	100	
ΔPPENIDIX 2· SLIRVEY HEΔI TH & SΔEETV	175	

LIST OF TABLES

Table 1: Classified Vehicle Count Survey Specifications	5
Table 2: Unclassified Pedestrian / Cyclist Count Survey Specifications	
Table 3: Survey Peak Hour	21
Table 4: Survey Peak Hour (Pedestrian / Cyclist)	
Table 5: Queue Length Survey Results	
Table 6: On-street Parking Occupancy Survey Results	65
Table 7: Lebuh Pantai Multi-storey Parking	79
Table 8: Tun Syed Off Street Parking	79
Table 9: Union Off Street Parking	79
Table 10: Parking Dwell Time Survey Results	
Table 11: Zone Numbering and Description	85
Table 12: Unit Matrix	
Table 13: TomTom Matrix	87
Table 14: Deliverable Stages	9.0

LIST OF FIGURES

Figure 1: Classified Vehicle Count Survey Junctions	6
Figure 2: Unclassified Pedestrian / Cyclist Count Survey Junctions	7
Figure 3: Junction Layout (Pre-collected before On-site Verification)	8
Figure 4: Road Movement Direction	
Figure 5: Queue Lenght Survey Junctions	10
Figure 6: On-street Parking Survey Locations	12
Figure 7: Off-street Parking Survey Locations	14
Figure 8: Example of GPS Data Collection Process	16
Figure 9: Existing Junctions Surveyed in Study Area	20
Figure 10: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Full	22
Figure 11: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Northwest Section	23
Figure 12: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Northeast Section	24
Figure 13: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Southwest Section	25
Figure 14: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Southeast Section	26
Figure 15: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Full	27
Figure 16: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Northwest Section	28
Figure 17: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Northeast Section	29
Figure 18: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Southwest Section	30
Figure 19: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Southeast Section	31
Figure 20: Existing Pedestrian / Cyclist Crossings Surveyed in Study Area	32
Figure 21: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Full	33
Figure 22: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Northwest Section	34
Figure 23: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Northeast Section	35
Figure 24: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Southwest Section	36
Figure 25: 2021 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Southeast Section	37
Figure 26: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Full	38
Figure 27: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Northwest Section	
Figure 28: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Northeast Section	40
Figure 29: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Southwest Section	
Figure 30: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Southeast Section	42
Figure 31: Junction Layout (After On-site Verification)	43
Figure 32: Junction Signal Location	
Figure 33: Surveyed Signalized Junctions	
Figure 34: Signal Data for Junction between J3 &J4	
Figure 35: Signal Data for Junction 23	47
Fig. 20 City ID 12 Co. L. City 24	10

Figure 37: On-str	reet Parking Occupancy Survey Locations	64		
	Off-street Parking Survey Locations			
Figure 39: Parkin	Parking Dwell Time Survey Locations			
Figure 40: Survey	y Zoning System	84		
Figure 41: Data (Categories	90		
Figure 42: Examp	igure 42: Example of PCU Conversion			
Figure 43: Examp	ole of Peak Hour Identification	93		
	ole of Flow Balance Comparison			
Figure 45: Examp	ole of OD Table Data Source	95		
	on 1 Layout			
Figure 46: Explar	nation of symbology	100		
Figure 48: Junction	on 1 Trafic Condition	100		
Figure 49: Junction	on 2 Layout	101		
Figure 50: Junction	on 2 Traffic Condition	101		
Figure 51: Junction	on 3 Layout	102		
	on 3 Traffic Condition			
Figure 53: Junction	on 4 Layout	103		
Figure 54: Junction	on 4 Traffic Condition	103		
Figure 55: Junction	on 5 Layout	104		
	on 5 Traffic Condition			
Figure 57: Junction	on 6 Layout	105		
Figure 58: Junction	on 6 Traffic Condition	105		
Figure 59: Junction	on 7 Layout	106		
Figure 60: Junction	on 7 Traffic Condition	106		
Figure 61: Junction	on 8 Layout	107		
	on 8 Traffic Condition			
Figure 63: Junction	on 9 Layout	108		
	on 9 Traffic Condition			
Figure 65: Junction	on 10 Layout	109		
Figure 66: Junction	on 10 traffic Condition	109		
Figure 67: Junction	on 11 Layout	110		
Figure 68: Junction	on 11 traffic Condition	110		
Figure 69: Junction	on 12 Layout	111		
Figure 70: Junction	on 12 traffic Condition	111		
Figure 71: Junction	on 13 Layout	112		
Figure 72: Junction	on 13 Traffic Condition	112		
Figure 73: Junction	on 14 Layout	113		
Figure 74: Junction	on 14 traffic Condition	113		
Figure 75: Junction	on 15 Layout	114		
Figure 76: Junction	on 15 Traffic Condition	114		
Figure 77: Junction	on 16 Layout	115		

rigure 76. Junction 16 Tranic Condition	
Figure 79: Junction 17 Layout	116
Figure 80: Junction 17 Traffic Condition	
Figure 81: Junction 18 Layout	
Figure 82: Junction 18 Traffic Condition	117
Figure 83: Junction 19 Layout	118
Figure 84: Junction 19 Traffic Condition	118
Figure 85: Junction 20 Layout	
Figure 86: Junction 20 traffic Condition	119
Figure 87: Junction 21 Layout	120
Figure 88: Junction 21 Traffic Condition	120
Figure 89: Junction 22 Layout	
Figure 90: Junction 22 traffic Condition	121
Figure 91: Junction 23 Layout	122
Figure 92: Junction 23 traffic Condition	122
Figure 93: Junction 24 Layout	123
Figure 94: Junction 24 traffic Condition	123
Figure 95: Junction 25 Layout	124
Figure 96: Junction 25 traffic Condition	124
Figure 97: Junction 26 Layout	125
Figure 98: Junction 26 traffic Condition	125
Figure 99: Junction 27 Layout	126
Figure 100: Junction 27 Traffic Condition	126
Figure 101: Junction 28 Layout	127
Figure 102: Junction 28 traffic Condition	127
Figure 103: Junction 29 Layout	128
Figure 104: Junction 29 traffic Condition	
Figure 105: Junction 30 Layout	129
Figure 106: Junction 30 Traffic Condition	
Figure 107: Junction 31 Layout	130
Figure 108: Junction 31 Traffic Condition	130
Figure 109: Junction 32 Layout	131
Figure 110: Junction 32 Traffic Condition	131
Figure 111: Junction 33 Layout	132
Figure 112: Junction 33 Traffic Condition	132
Figure 113: Junction 34 Layout	
Figure 114: Junction 34 Traffic Condition	
Figure 115: Junction 35 Layout	
Figure 116: Junction 35 Traffic Condition	
Figure 117: Junction 36 Layout	
Figure 118: Junction 36 Traffic Condition	

Figure 119. Junction 37 Layout	
Figure 120: Junction 37 Traffic Condition	136
Figure 121: Junction 38 Layout	137
Figure 122: Junction 38 Traffic Condition	137
Figure 123: Junction 39 Layout	
Figure 124: Junction 39 Traffic Condition	138
Figure 125: Junction 40 Layout	139
Figure 126: Junction 40 Traffic Condition	139
Figure 127: Junction 41 Layout	
Figure 128: Junction 41 Traffic Condition	
Figure 129: Junction 42 Layout	141
Figure 130: Junction 42 Traffic Condition	141
Figure 131: Junction 43 Layout	
Figure 132: Junction 43 Traffic Condition	142
Figure 133: Junction 44 Layout	143
Figure 134: Junction 44 Traffic Condition	143
Figure 135: Junction 45 Layout	144
Figure 136: Junction 45 Traffic Condition	144
Figure 137: Junction 46 Layout	145
Figure 138: Junction 46 Traffic Diagram	145
Figure 139: Junction 47 Layout	146
Figure 140: Junction 47 Traffic Diagram	146
Figure 141: Junction 48 Layout	
Figure 142: Junction 48 Traffic Condition	147
Figure 143: Junction 49 Layout	148
Figure 144: Junction 49 Traffic Condition	
Figure 145: Junction 50 Layout	
Figure 146: Junction 50 Traffic Condition	149
Figure 147: Junction 51 Layout	150
Figure 148: Junction 51 Traffic Condition	150
Figure 149: Junction 52 Layout	151
Figure 150: Junction 52 Traffic Condition	151
Figure 151: Junction 53 Layout	152
Figure 152: Junction 53 Traffic Condition	152
Figure 153: Junction 54 Layout	
Figure 154: Junction 54 Traffic Condition	
Figure 155: Junction 55 Layout	154
Figure 156: Junction 55 Traffic Condition	154
Figure 157: Junction 56 Layout	155
Figure 158: Junction 56 Traffic Condition	155
Figure 159: Junction 57 Layout	156

Figure 160: Junction 57 Traffic Condition	156
Figure 161: Junction 58 layout	
Figure 162: Junction 58 Traffic Condition	
Figure 163: Junction 59 Layout	
Figure 164: Junction 59 Traffic Condition	
Figure 165: Junction 60 Layout	159
Figure 166: Junction 60 Traffic Condition	159
Figure 167: Junction 61 Layout	160
Figure 168: Junction 61 Traffic Condition	160
Figure 169: Junction 62 Layout	161
Figure 170: Junction 62 Traffic Condition	161
Figure 171: Junction 63 Layout	162
Figure 172: Junction 63 Traffic Condition	162
Figure 173: Junction 64 Layout	163
Figure 174: Junction 64 Traffic Condition	163
Figure 175: Junction 65 Layout	164
Figure 176: Junction 65 Traffic Condition	
Figure 177: Junction 66 Layout	165
Figure 178: Junction 66 Traffic Condition	
Figure 179: Junction 67 Layout	
Figure 180: Junction 67 Traffic Condition	
Figure 181: Junction 68 Layout	
Figure 182: Junction 68 Traffic Condition	
Figure 183: Junction 69 Layout	
Figure 184: Junction 69 Traffic Condition	
Figure 185: Junction 70 Layout	
Figure 186: Junction 70 Traffic Condition	
Figure 187: Junction 71 Layout	
Figure 188: Junction 71 Traffic Condition	170
Figure 189: Junction 72 Layout	171
Figure 190: Junction 72 Traffic Condition	171
Figure 191: Junction 73 Layout	172
Figure 192: Junction 73 Traffic Condition	
Figure 193: Junction 74 Layout	
Figure 194: Junction 74 Traffic Condition	173
Figure 195: Junction 75 Layout	174
Figure 196: Junction 75 Traffic Condition	174

INTRODUCTION

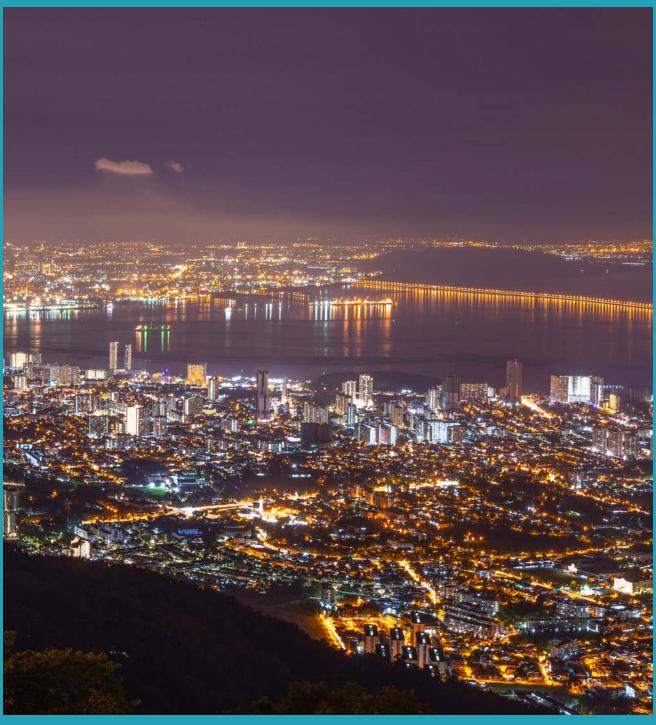


Photo: Adobe Stock

1.1 BACKGROUND

Ramboll has been engaged through the ASEAN Australia Smart Cities Trust Fund (AASCTF) to conduct a Pilot Project for Penang. This Pilot Project intervention involves the development of a Transport Micro-Simulation Model of the historical centre of Georgetown that can be used to assess future mobility interventions such as public transport, traffic improvements, pedestrianization and cycling improvements.

This Pilot Project involves the development and calibration of the micro-simulation model using PTV Vissim software and testing of a limited set of potential future interventions for Georgetown, as well as training of Digital Penang / MBPP staff in the use of PTV Vissim.

This report outlines the project survey details including survey methodologies and results. Traffic surveys included video Traffic Count surveys, Parking Surveys, and Origin-Destination surveys.

Survey results will be used in the development and calibration of the micro-simulation model to provide an accurate representation of the transportation environment in Georgetown.

1.2 SURVEY OBJECTIVES

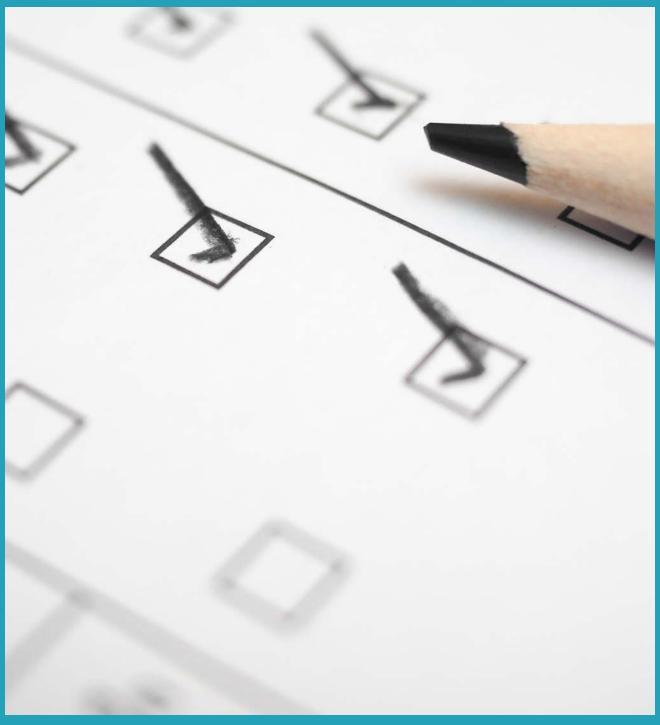
Surveys proposed for this project are intended to establish solid ground for traffic planning and simulation works ahead. Quality survey data is vital to ensure the traffic analysis is based on the real-world situation and reflect what is happening on-site accurately. In order to do this, a detailed survey plan has been proposed and carried out by Ramboll with information presented in the chapters below.

1.3 ORGANISATION OF REPORT

Following this introduction, the report is structured as follows:

- Chapter 2 provides an overview of the survey methodology, including the methodologies for traffic surveys, parking surveys and origin-destination data survey,
- Chapter 3 presents the results from traffic surveys which contains results in classified vehicle count, unclassified pedestrian / cyclist count and junction layout and signal surveys,
- Chapter 4 contains the results from parking surveys including on-street parking occupancy, off-street parking occupancy, and parking dwell time survey,
- Chapter 5 includes the results from origin-destination data survey, including unit matrix and result matrices from the survey,
- Chapter 6 describes how the survey data were utilised from on-site results to model inputs of this study, and
- Chapter 7 provides the next steps of the study.

SURVEY METHODOLOGY



This chapter outlines the survey methodology for data collection needed to develop the micro-simulation model. On-site traffic surveys have been undertaken in November 2021 by the project sub-contractor, Fox Traffic Sdn. Bhd. In the meantime, the origin-destination data was collected from GPS data provider TomTom for the same duration of the on-site data extraction.

2.1 SURVEY INTRODUCTION

To develop a representative simulation of transportation in Georgetown for this study, accurate and comprehensive multi-modal transport data is required to be collected across the network with a combination of on-site video capture and remote GPS data collection.

Survey data came from two sources for this project, namely video traffic surveys and GPS data collection. On-site video traffic surveys allow us to capture an accurate and highly detailed snapshot of traffic movement around every junction and parking area, disaggregated across each peak period. GPS data collection gives us a 'birds eye view' of movement across the network, aggregated into weeks or months of travel patterns, travel times and speeds across a collective group of user experiences. Bringing these two data sources together gives us an accurate picture of transport patterns to develop and calibrate a simulation model that is representative of on-site conditions.

The following sections describe the details of survey methodology for each type of survey.

2.2 TRAFFIC SURVEYS

For video-based traffic surveys, classified movement counts were conducted during peak morning periods (7:00am to 10:00am) and peak afternoon/evening periods (4:30pm to 7:30pm) for the following dates:

- 9 November 2021 (Tuesday),
- 10 November 2021 (Wednesday), and
- 11 November 2021 (Thursday)

'Classified' refers to counting separately the different vehicle classes including motorbikes, cars, light goods vehicles, heavy goods vehicles, buses etc. The following sections outline the survey specifications for this study.

2.2.1 CLASSIFIED VEHICLE COUNT SURVEYS

The specifications for the classified vehicle count survey are shown in Table 1. All junction surveys were disaggregated into the various classifications for each turning movement at the junction separately.

Table 1: Classified Vehicle Count Survey Specifications

Item	Specifications	
Survey Locations	Junctions shown in Figure 1	
Survey Classifications	Cars, Taxis, Motorcycles, Light Goods Vehicles & Small Vans, Heavy Goods Vehicles with 3 axles and above, Buses	
Aggregation	All counts to be in 15-minute intervals	
Queues	Maximum observed queue lengths should be recorded for each junction approach in 15-minute intervals	
Signal Phase and Timings	Junction signal phase and timings should be taken from video recordings of the signal for a minimum of continuous 15 minutes in each one-hour of junction survey. Recorded signal phases, timings and videos are required to be submitted.	
Survey Days	9 November 2021 (Tuesday), 10 November 2021 (Wednesday), and 11 November 2021 (Thursday);	
Survey Time Periods	07:00 – 10:00 and 16:30 – 19:30	

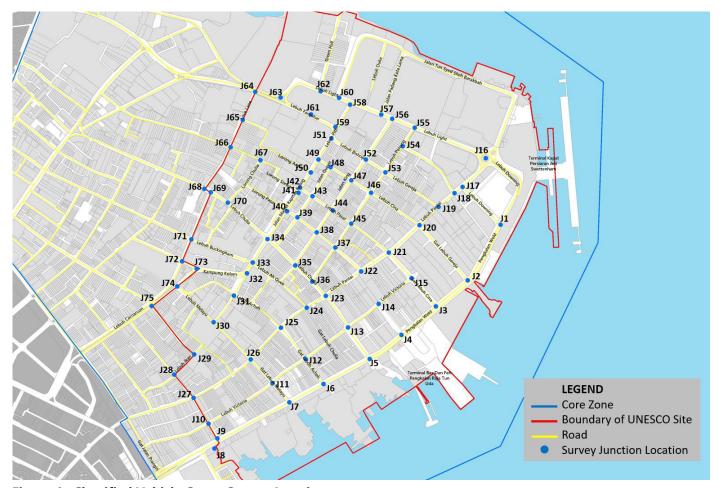


Figure 1: Classified Vehicle Count Survey Junctions

The survey has been carried out from 9 November 2021 to 11 November 2021. It is observed that during this week traffic volume has picked up following the ending of MCO* period in Penang state.

^{*}MCO refers to Movement Control Order, a directive from the Malaysian central government restricting movement of persons within or between states to limit the spread of Covid-19.

2.2.2 UNCLASSIFIED PEDESTRIAN / CYCLIST COUNT SURVEYS

During the same date and time duration of vehicle count survey, number of pedestrians and cyclists crossing the identified junctions was also surveyed to provide a comprehensive picture of the pedestrian and cyclist demand throughout the study area.

Pedestrian and cyclists were recorded at crossing points throughout the road network when they were crossing the street. The number for pedestrians and cyclists is unclassified, which means the results are in single combined class without further differentiation of user profiles (such as students, elderly, etc.). This approach was selected as it provide sufficient basis to evaluate the impact of pedestrian and cyclist movement to an overall traffic network, which suits the purpose of this report.

Further details of the unclassified pedestrian / cyclist survey are presented in Table 2 and Figure 2.

Table 2: Unclassified Pedestrian / Cyclist Count Survey Specifications

Item	Specifications	
Survey Locations	Pedestrians and cyclists crossing each arm of junctions shown in Figure 2	
Survey Classifications	Single Class	
Aggregation	All counts to be in 15-minute intervals	
Survey Days	9 November 2021 (Tuesday), 10 November 2021 (Wednesday), and 11 November 2021 (Thursday);	
Survey Time Periods	07:00 – 10:00 and 16:30 – 19:30	



Figure 2: Unclassified Pedestrian / Cyclist Count Survey Junctions

2.2.3 JUNCTION LAYOUT AND SIGNAL SURVEYS

For traffic analysis and modeling, an accurate representation of network inventory is vital to the realistic reproduction of traffic conditions on-site.

As a part of this survey exercise, a full record inventory has been made of existing junction and road layouts, turning movements allowed or banned, traffic signal information, traffic lane configurations, bus stops, internal parking, and public transport facilities, waiting and loading restrictions, and general site layout.

Junction layout and traffic signals have been pre-collected and verified on-site with actual situation during the site works from 9 to 11 November 2021. Figure 3 shows the junction layout before on-site verification.



Figure 3: Junction Layout (Pre-collected before On-site Verification)

9 SURVEY METHODOLOGY

In addition, the junction layout and network inventory information also includes the direction of travel along roads within the study which are shown in Figure 4 below.



Figure 4: Road Movement Direction

Further survey results with on-site verification are presented in Chapter 3.

2.2.4 QUEUE LENGTH SURVEYS

Together with classified vehicle traffic counts as described in Section 2.2.1, queue length counts were also conducted during the site survey.

Queue length is an important indicator of junction performance in an urban traffic scenario. Vehicle queue is formed from delays at junction including geometry delays and signal delays. It is also a reflection of the reaction time drivers have toward traffic regulations.

Queue length information was collected at the locations shown in Figure 5 during the site survey.

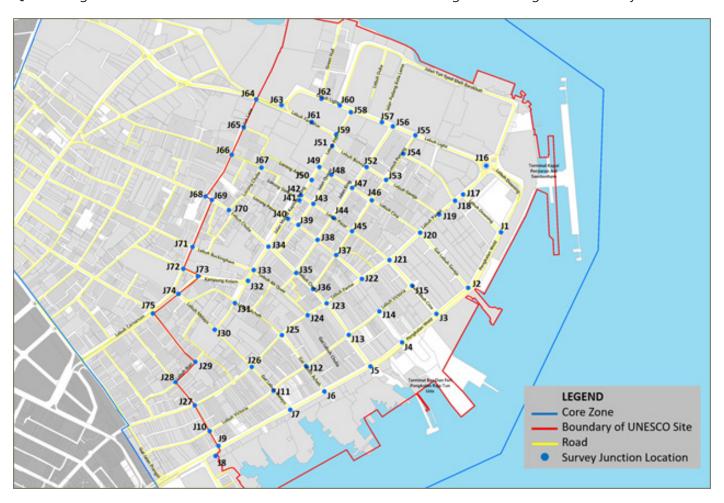


Figure 5: Queue Lenght Survey Junctions

Further survey results and their usage for site calibration were presented in Chapter 3.

2.3 PARKING SURVEYS

Parking surveys have been conducted at both on-street and off-street parking facilities. On-street parking was classified by street and midblock section. Illegal parking was also recorded. The parking surveys were meant to gain an understanding of not just traffic flowing through the historical centre of Georgetown, but also parking behavior within the study area.

For on-street parking, the following information has been captured:

- A. On-street Photos by survey team: Car Park Occupancy by one-hour segments of all on-street parking across the survey period.
- B. On-street video surveys: Sampled arrival/departure counts and parking dwell time, by vehicle classification, at selected locations, for a period of 15 minutes within each one-hour survey period.

The following figure identifies the on-street (yellow) parking areas included within the parking occupancy survey (A). Selected locations for on-street parking arrival, departure, dwell time survey (B) are also indicated in purple boxes in Figure 6.

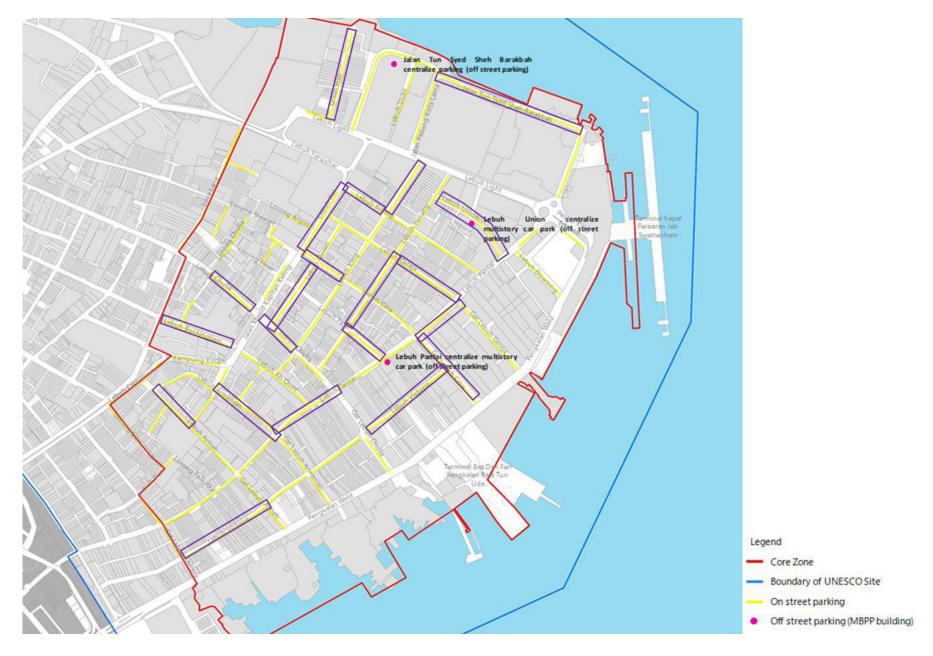


Figure 6: On-street Parking Survey Locations

Parking surveys have been conducted in conjunction with the surrounding junction surveys.

For off-street parking, the following information was collected:

- A. Through video recording: Arrival and departure counts, by vehicle classification
- B. Car Park Occupancy across the survey period

Figure 7 identifies the off-street parking areas marked in pink points that are to be included within the parking occupancy survey.



Figure 7: Off-street Parking Survey Locations

Further survey results from parking surveys are presented in Chapter 4.

2.4 ORIGIN-DESTINATION DATA SURVEY

GPS data collected for this study refers to historic data and real time data within the road network based on GPS location of vehicles on-site. This data was collected and aggregated by GPS navigation provider TomTom. There are two types of GPS data to be used for this study, which are travel pattern data and speed data.

Using travel pattern data, in-depth information about the distribution of the motorized traffic from various locations can be extracted to calibrate the transport models and simulations, and to better understand travel patterns.

Through speed data, bottleneck analysis of the road network can be assessed to identify significant speed reductions during peak hours, and to identify potential traffic safety issues.

The GPS data acquired to develop and calibrate the micro-simulation model includes the following components:

- Origin-destination travel pattern data; and
- Travel time and speed data

2.4.1 ORIGIN-DESTINATION TRAVEL PATTERN DATA COLLECTION

Origin-Destination Travel Pattern Data can be used to approximate the travel patterns within the model area for more accurate representation of the on-site condition. To generate this data, results from an online data platform named "TomTom O/D Analysis" was used to provide trip investigation based on a high volume of location data.

TomTom O/D Analysis uses advanced algorithms to analyze anonymized Floating Car Data (FCD) from 600+ million connected devices – providing the project with the authoritative view of what's happening on the road. TomTom gathers real-time FCD by combining measurements of existing infrastructure with signals from anonymous TomTom connected GPS devices. TomTom also archives this data to create a historical traffic database.

All major entry and exit points to the study area were used for collection of origin-destination travel pattern data for this simulation model. An example of the data collection process is shown in Figure 8.

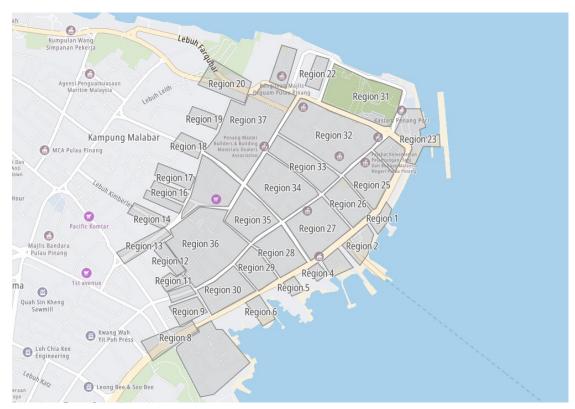


Figure 8: Example of GPS Data Collection Process

TRAVEL TIME & SPEED DATA COLLECTION

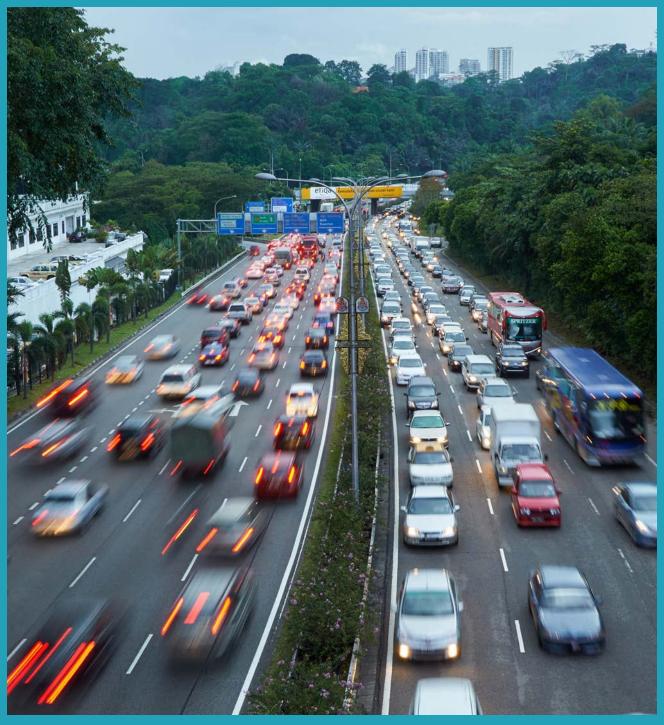
Travel Time & Speed Data allows the modeled results to be compared with real-life data for further model calibration. Within this data, two types of analysis are used for this study:

- Route Analysis: to define a specific route and generate average speeds, average travel times and sample size (number of vehicles that traversed a segment).
- Speed & Density Analysis: focused analysis on sample size (number of vehicles that traversed a segment), including speed and density of the sampled vehicles.

Major arterial roads within the study area were used for data collection of travel time and speed information. The travel time and speed data collection occurred at the same time as the traffic survey to ensure consistency between datasets.

Datasets were compiled to aggregate data analysed over one day.

TRAFFIC SURVEYS



3.1 **SURVEY TASKS**

As described in the survey methodology sections, on-site video-based traffic surveys were conducted during peak morning periods (7:00 am to 10:00 am) and peak afternoon/evening periods (4:30 pm to 7:30 pm) on 9 November 2021 (Tuesday), 10 November 2021 (Wednesday), and 11 November 2021 (Thursday). The survey aimed to produce results for the following tasks:

- 1. Classified vehicles count surveys
- 2. Unclassified pedestrian / cyclist count surveys
- 3. Junction layout and signal surveys

Results from each of the tasks are shown in Section 3.2.1 to Section 3.2.3 below.

3.2 **SURVEY RESULTS**

3.2.1 CLASSIFIED VEHICLE COUNT SURVEYS

As proposed and presented to the client, traffic counts at existing junctions were conducted to obtain the current background road network demand.

The junction traffic counts were conducted at the following seventy-five locations:

- Junction 1: Pengkalan Road/Lebuh Downing
- Junction 2: Pengkalan Road/Gat Lebuh Gereja
- Junction 3: Pengkalan Road/Gat Lebuh China
- · Junction 4: Pengkalan Road/Gat Lebuh Pasar
- Junction 5: Pengkalan Road/Gat Lebuh Chulia
- Junction 6: Pengkalan Road/Gat Lebuh Armenian
- Junction 7: Pengkalan Road/Gat Lebuh Acheh
- Junction 8: Pengkalan Road/Lintasan Pengkalan 1
- Junction 9: Pengkalan Road/Gat Lebuh Melayu
- Junction10: Lebuh Victoria/ Gat Lebuh Melayu
- Junction11: Lebuh Victoria/ Gat Lebuh Acheh
- Junction12: Lebuh Victoria/ Gat Lebuh Armenian
- Junction13: Lebuh Victoria/ Gat Lebuh Chulia
- Junction14: Lebuh Victoria/ Gat Lebuh Pasar
- Junction15: Lebuh Victoria/ Gat Lebuh China
- Junction16: Lebuh Pantai/ Pesara King Edward
- Junction17: Lebuh Pantai/ Lebuh Downing
- Junction18: Beach Street/ Lebuh Union
- Junction19: Beach Street/ Bishop Street
- Junction20: Beach Street/ Gat Lebuh Gereja
- Junction21: Beach Street/ Gat Lebuh China
- Junction22: Beach Street/ Gat Lebuh Pasar
- Junction23: Beach Street/ Gat Lebuh Chulia
- Junction24: Beach Street/ Lebuh Al Ouee
- Junction25: Beach Street/ Gat Lebuh Armenian

- Junction26: Beach Street/ Gat Lebuh Acheh
- Junction27: Beach Street/ Gat Lebuh Melayu
- Junction28: Lorong Ikan/ Lebuh Melayu
- Junction29: Lorong Toh Aka/ Lorong Carnavon
- Junction30: Lebuh Acheh/Lebuh Cannon
- Junction31: Lebuh Acheh/Lebuh Armenian
- Junction32: Jalan Masjid Kapitan Keling/Jalan Kampung Kolam
- Junction33: Jalan Masjid Kapitan Keling/Jalan Buckingham
- Junction34: Jalan Masjid Kapitan Keling/Chulia Street
- Junction35: Chulia Street/Lebuh King
- Junction36: Chulia Street/Lebuh penang
- Junction37: Lebuh Pasar/Penang Street
- Junction38: Lebuh Pasar/ Lebuh King
- Junction39: Lebuh Pasar/ Queen Street
- Junction40: Jalan Masjid Kapitan Keling/ Lebuh Pasar
- Junction41: Jalan Masjid Kapitan Keling/ Lorong Stewart
- Junction42: Jalan Masjid Kapitan Keling/ Lebuh China
- Junction43: Lebuh China/Queen Street
- · Junction44: Lebuh China/Lebuh King
- · Junction45: Lebuh China/Lebuh Penang
- Junction46: Lebuh Gereja /Lebuh Penang
- Junction47: Lebuh King /Church Street
- Junction48: Church Street/Queen Street
- Junction49: Jalan Masjid Kapitan Keling/ Church Street
- Junction50: Jalan Masjid Kapitan Keling/ Lorong Argus
- Junction51: Jalan Masjid Kapitan Keling/ Bishop Street
- Junction52: Bishop Street/Lebuh King
- Junction53: Bishop Street/Lebuh Penang
- Junction54: Lebuh Penang/Lebuh Union
- Junction55: Lebuh Penang/Light Street
- Junction56: Lebuh King/Light Street
- · Junction57: Lebuh Light/Jalan Padang Kota Lana
- Junction58: Lebuh Light/Jalan Masjid Kapitan Keling
- Junction59: Jalan Masjid Kapitan Keling/Lebuh Farquhar
- Junction60: Lebuh Light/Jalan Tun Syed Sheh Barakbah
- Junction61: Lebuh Farquhar/Local road
- Junction62: Lebuh Light/Jalan Green Hall
- Junction63: Lebuh Light/ Lebuh Farguhar
- Junction64: Lebuh Light/ Love Ln
- Junction65: Love Ln/Lorong Argus
- Junction66: Love Ln/Mountri Street
- Junction67: Lorong Stewart/Lorong Chulia
- Junction68: Love Ln/Chulia Street
- Junction69: Chulia Street/lebuh Carnavon
- Junction70: Chulia Street/lebuh Chulia
- Junction71: Lebuh Campbell/Lebuh Carnarvon
- Junction72: Pesara Claimant/Lebuh Carnarvon

- Junction73: Jalan Kampung Kolam/Lebuh Carnarvon
- Junction74: Lebuh Carnarvon/Lebuh Acheh
- Junction75: Lebuh Carnarvon/Lebuh Kimberley

The locations of seventy-five surveyed junctions are shown in Figure 9.



Figure 9: Existing Junctions Surveyed in Study Area

Traffic counts results were analysed to determine the peak 60-minute periods within the morning and evening peak periods. All traffic flows were converted and expressed in Passenger Car Units (PCUs). PCUs are factors that convert different classification of vehicles to be equivalent to a typical car. The following PCU factors were used (in accordance with Malaysian guidelines*) for the junction counts:

Car: 1.00Taxi: 1.00

Light Goods Vehicles (Lorry Kecil): 2.50Heavy Goods Vehicles (Lorry Besar): 3.00

• Bus: 3.00

• Motorcycle: 0.75

The peak hour traffic flows occurred during the times stated in Table 3.

Table 3: Survey Peak Hour

	Surveyed Time	Peak Hour Traffic
Weekday AM	07:00 to 10:00	08:15 to 09:15 (Traffic flows shown in Figure 10 to 14)
Weekday PM	16:30 to 19:30	17:00 to 18:00 (Traffic flows shown in Figure 15 to 19)

For the respective peak hours within the surveyed timings, the corresponding traffic flow volumes (in PCUs) in the background road network are shown in the following figures.



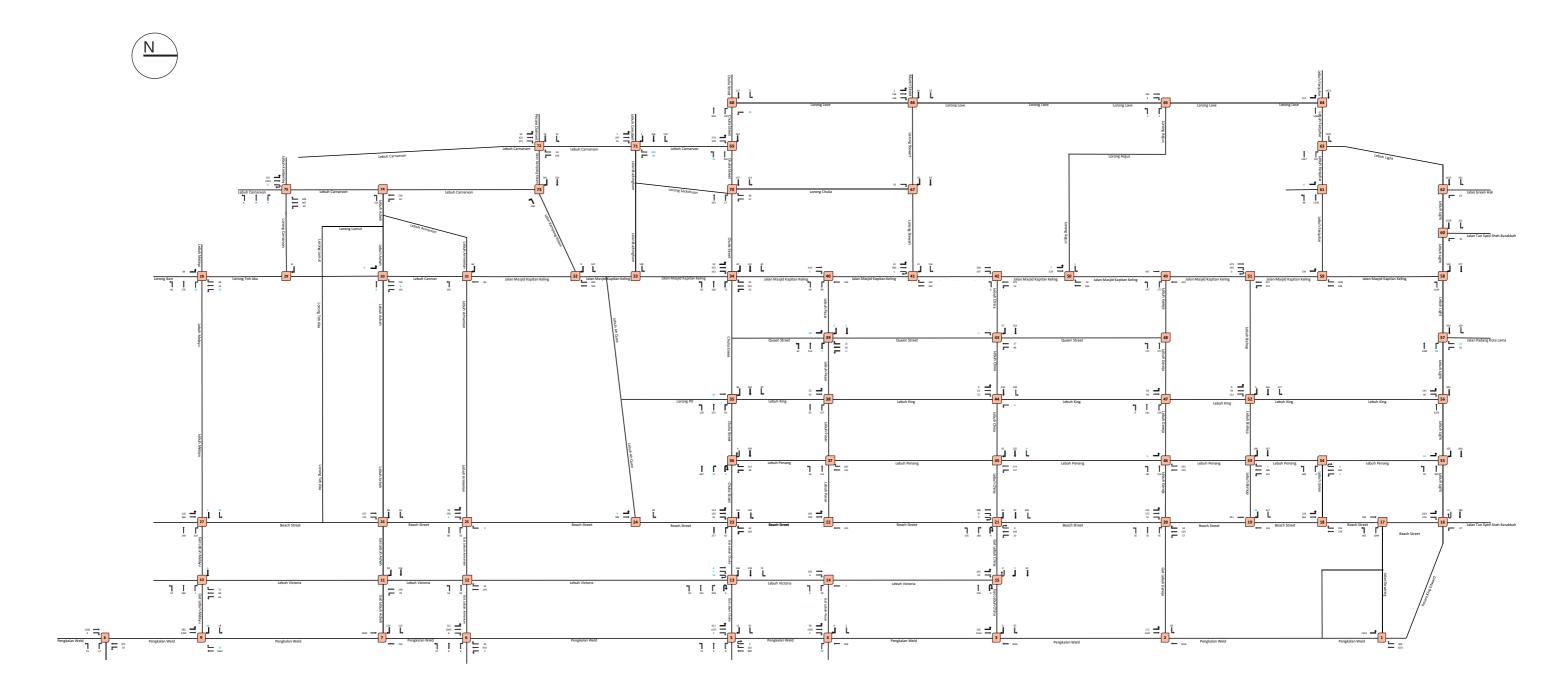


Figure 10: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak - Full

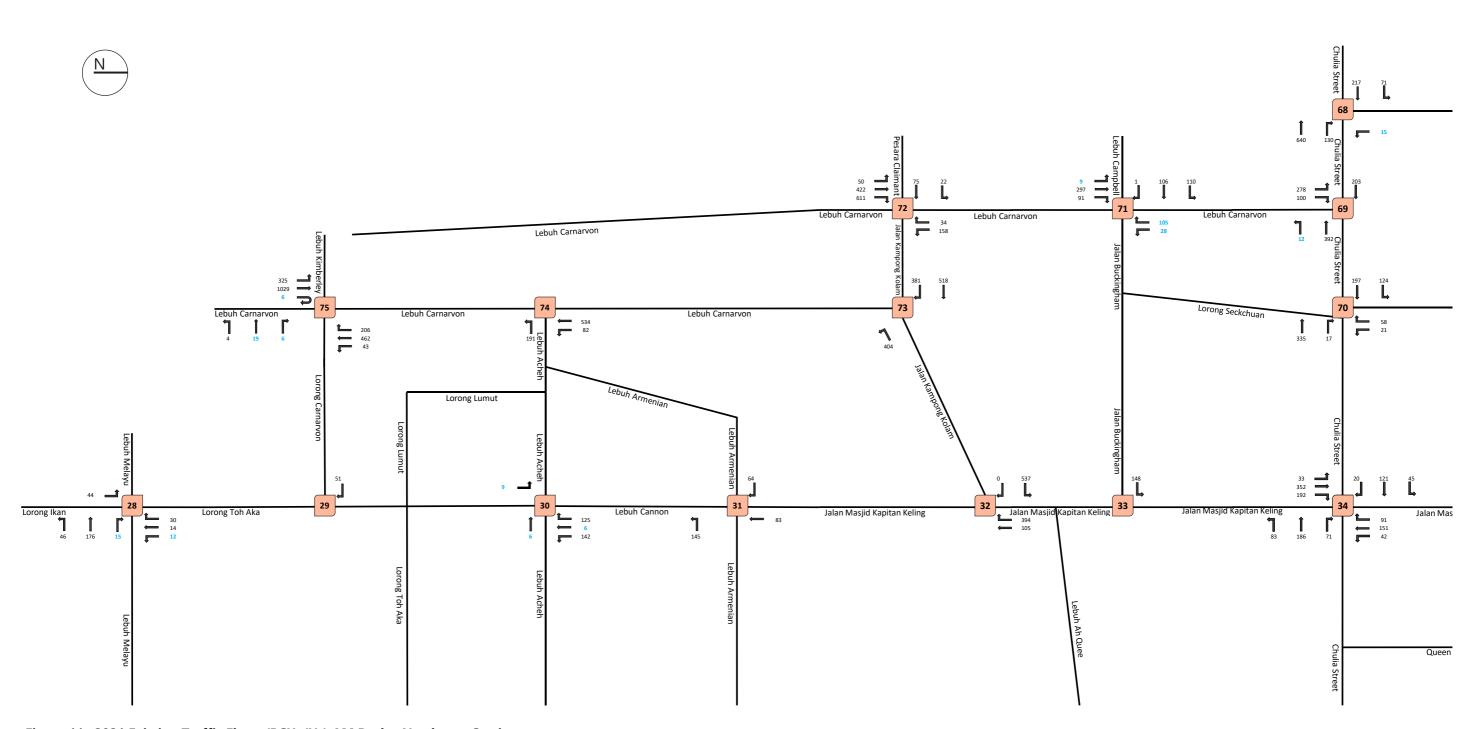


Figure 11: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak - Northwest Section

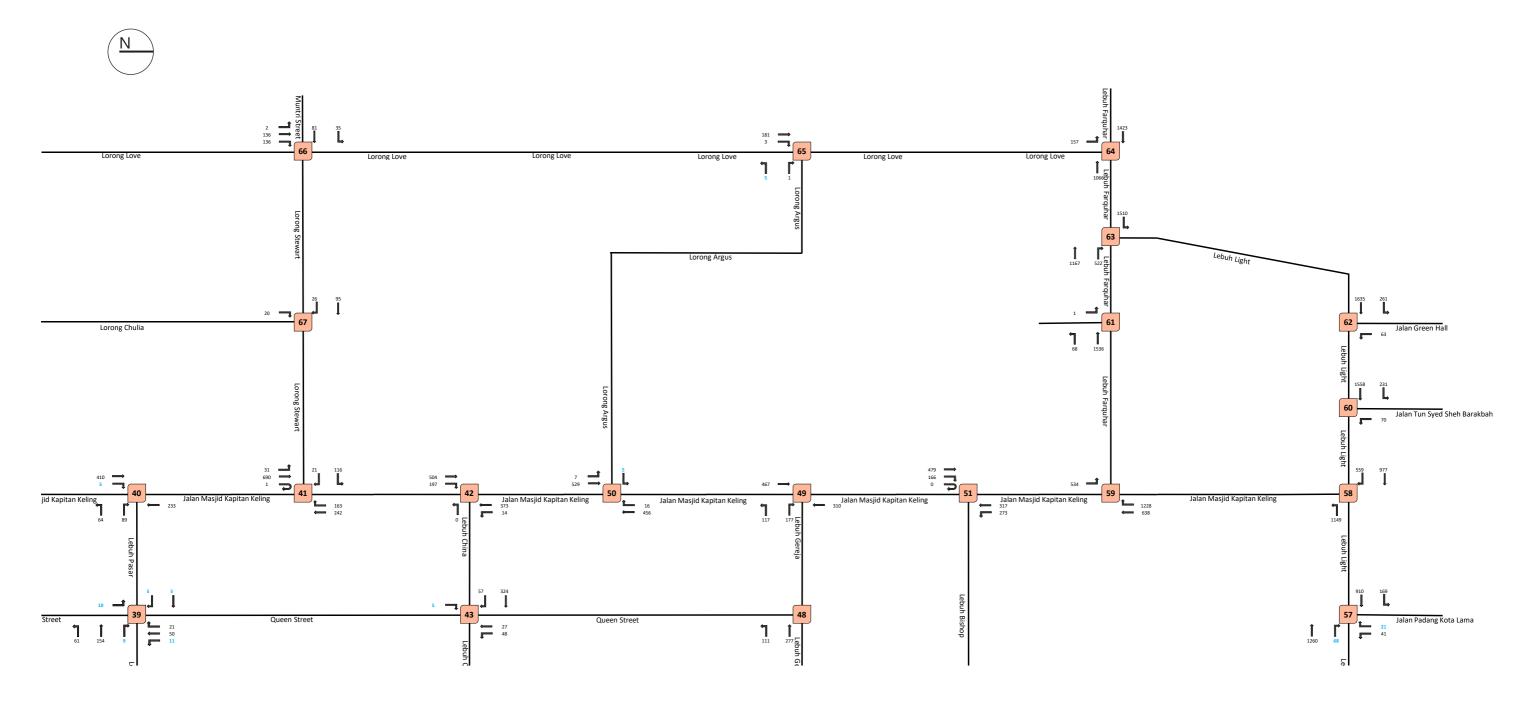


Figure 12: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Northeast Section

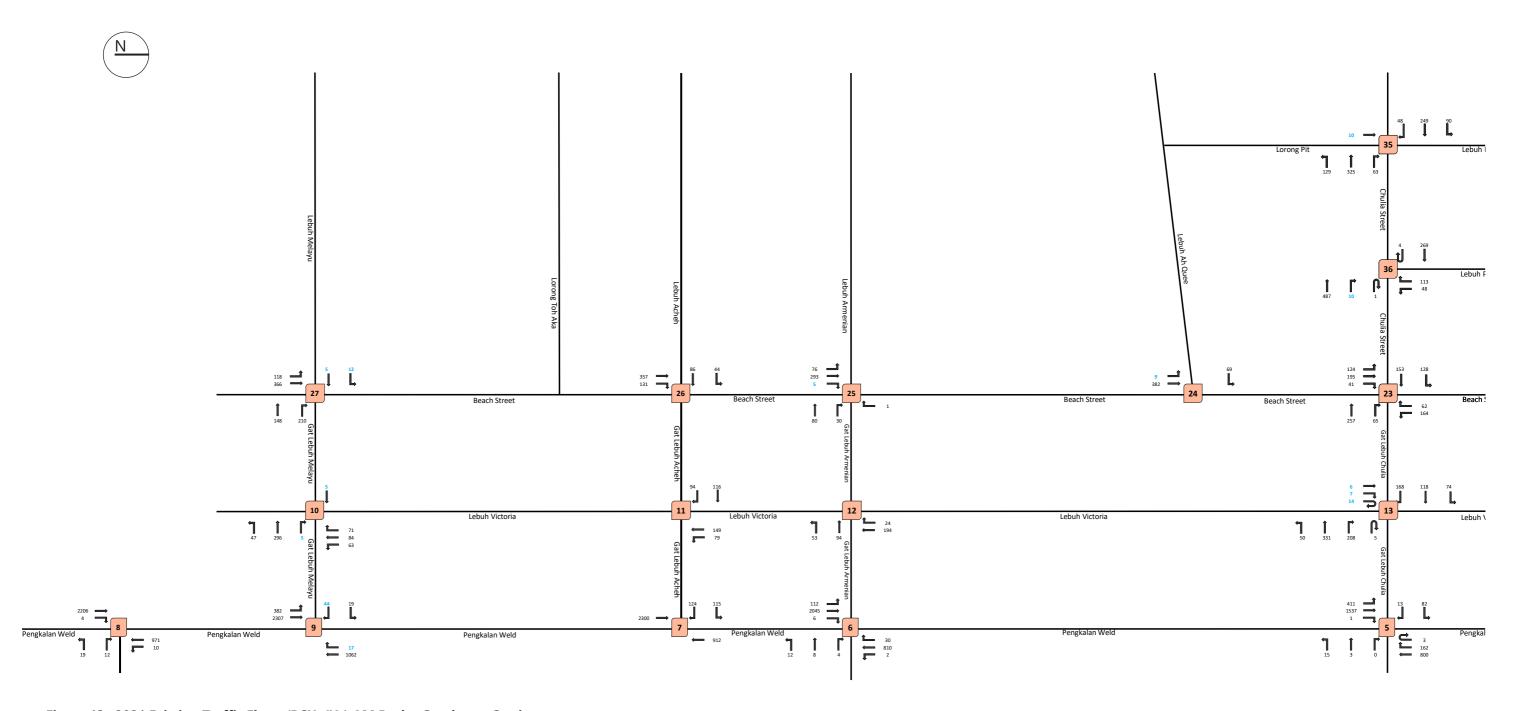


Figure 13: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Southwest Section

Pengkalan Weld

Pengkalan Weld

Figure 14: 2021 Existing Traffic Flows (PCUs/Hr) AM Peak – Southeast Section

Pengkalan Weld

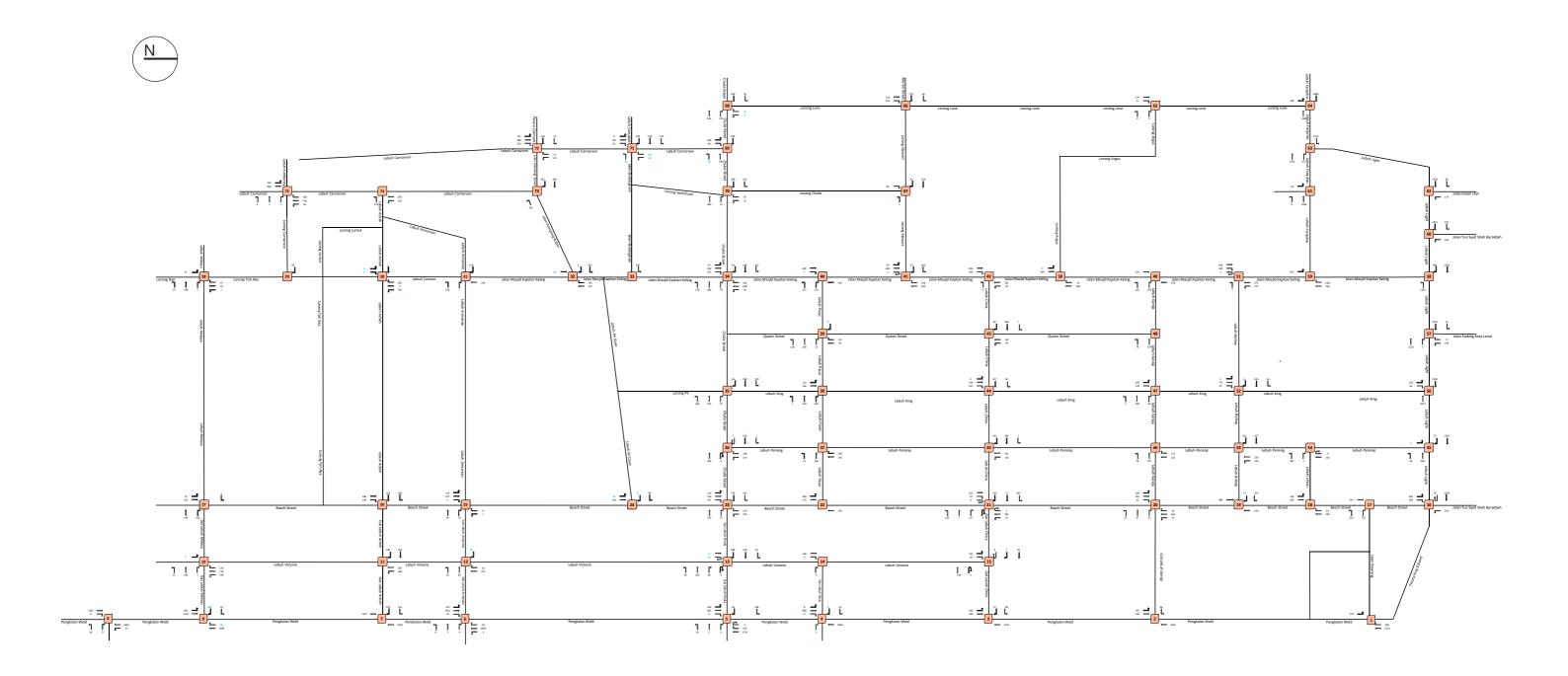


Figure 15: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Full

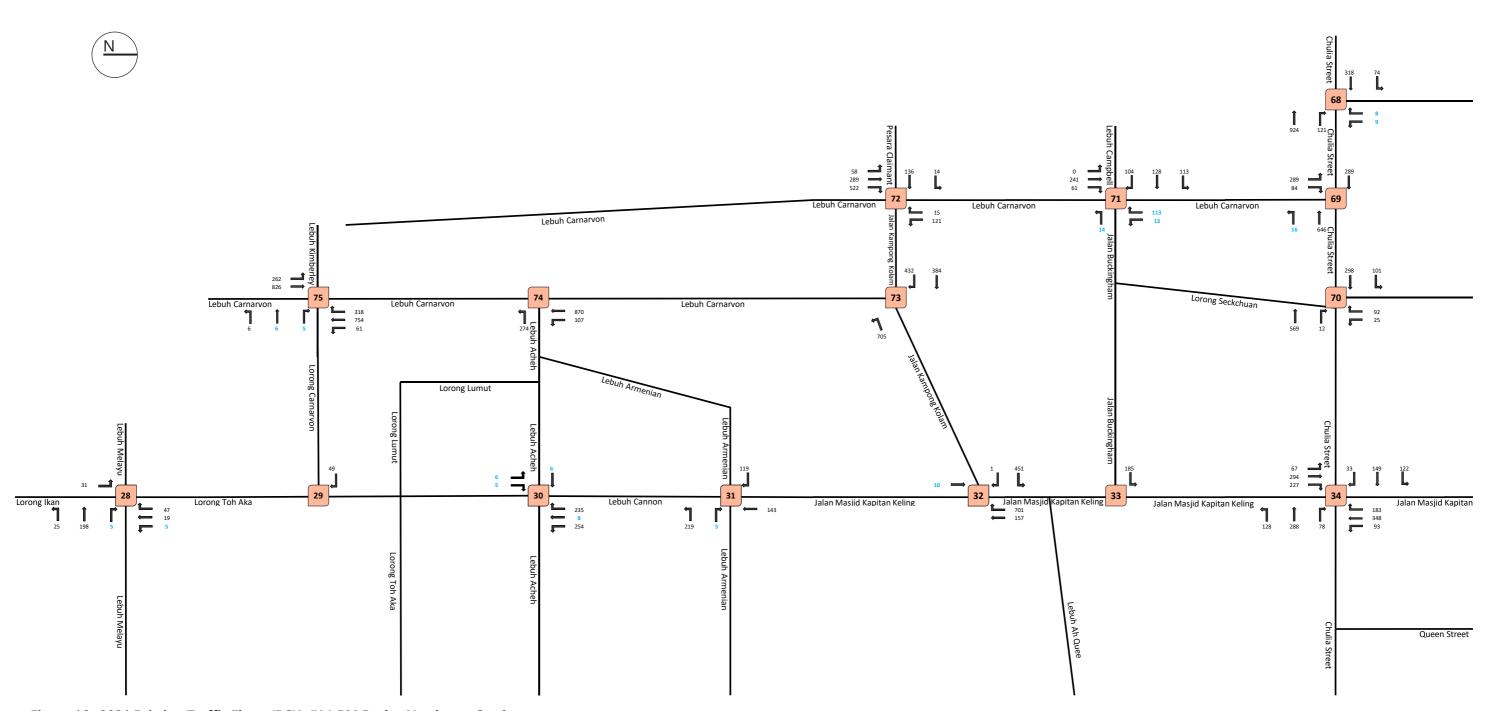


Figure 16: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Northwest Section

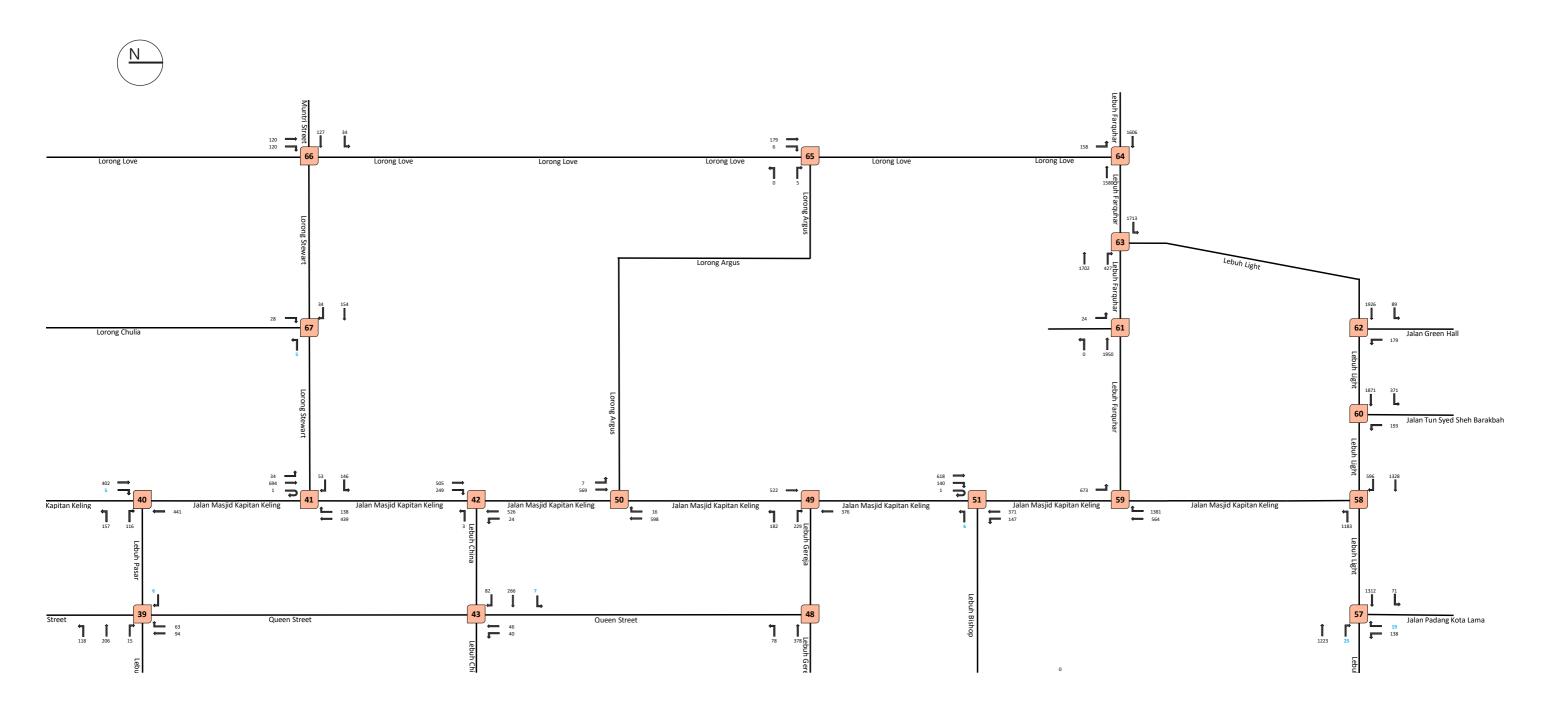


Figure 17: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Northeast Section

Pengkalan Weld

Figure 18: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Southwest Section

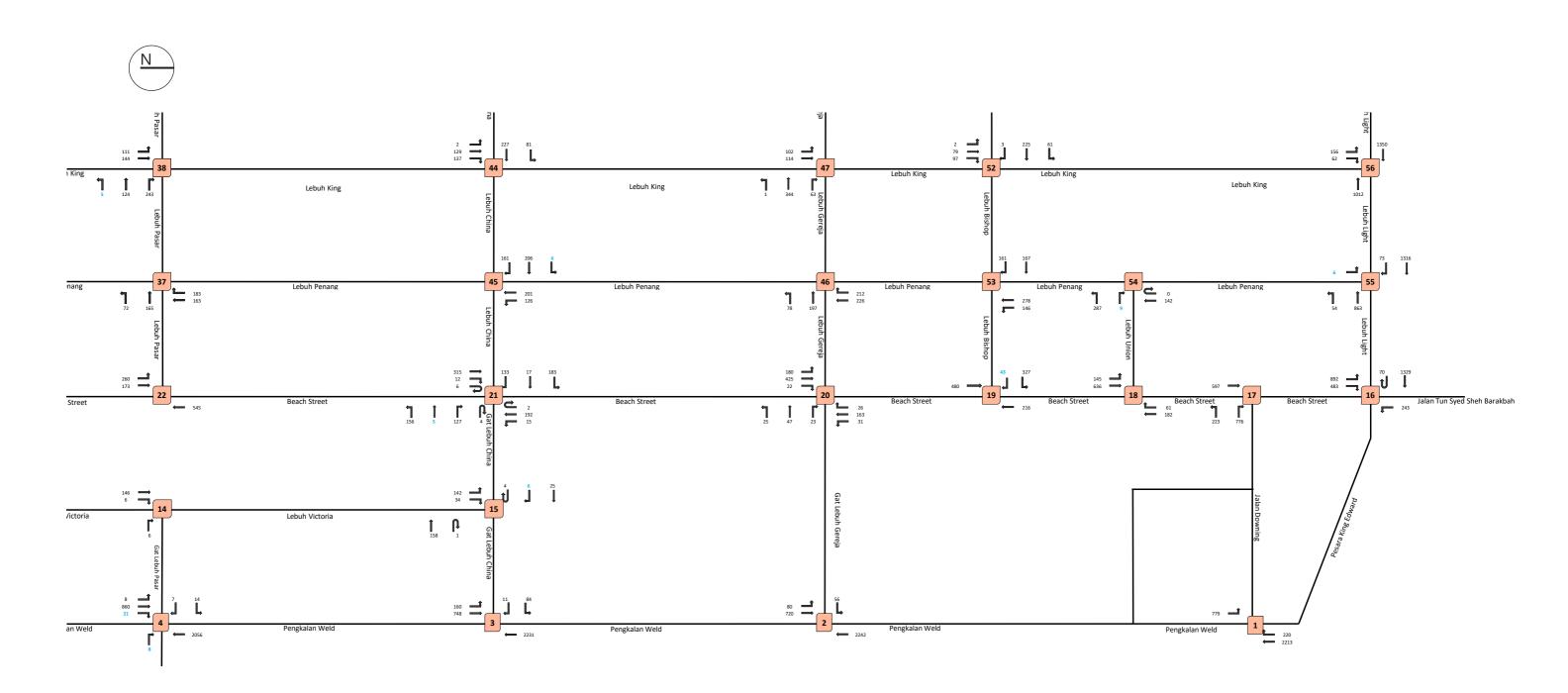


Figure 19: 2021 Existing Traffic Flows (PCUs/Hr) PM Peak – Southeast Section

3.2.2 UNCLASSIFIED PEDESTRIAN / CYCLIST COUNT SURVEYS

Pedestrian and cyclists were recorded at crossing points throughout the road network when they were crossing the street. The number for pedestrians and cyclists is unclassified, which means the results are in single combined class without further differentiation of user profiles (such as students, elderly, etc.). The locations of the seventy-five surveyed junctions are shown in Figure 20.

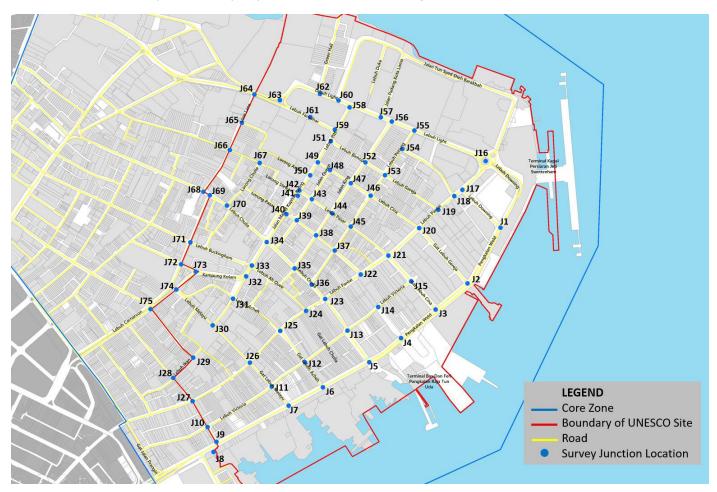


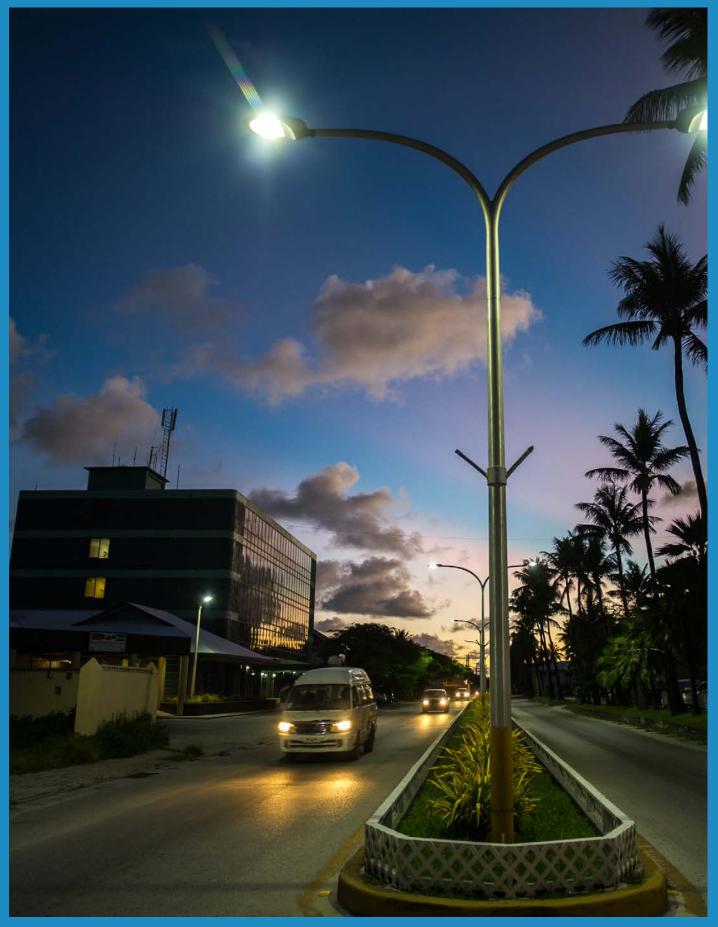
Figure 20: Existing Pedestrian / Cyclist Crossings Surveyed in Study Area

Pedestrian/cyclists count results were analysed to determine the peak 60-minute periods within the morning and evening peak periods. The peak hour pedestrian and cyclist flows follows the same period as traffic flows during the times stated in the table below.

Surveyed Time Peak Hour Pedestrian / Cyclists 08:15 to 09:15 Weekday AM 07:00 to 10:00 (Flows shown in Figure 21 to 25) 17:00 to 18:00 Weekday PM 16:30 to 19:30 (Flows shown in Figure 26 to 30)

Table 4: Survey Peak Hour (Pedestrian / Cyclist)

For the respective peak hours within the surveyed timings, the corresponding pedestrian / cyclist flow volumes in the road network are shown in Figures 21 to 30.



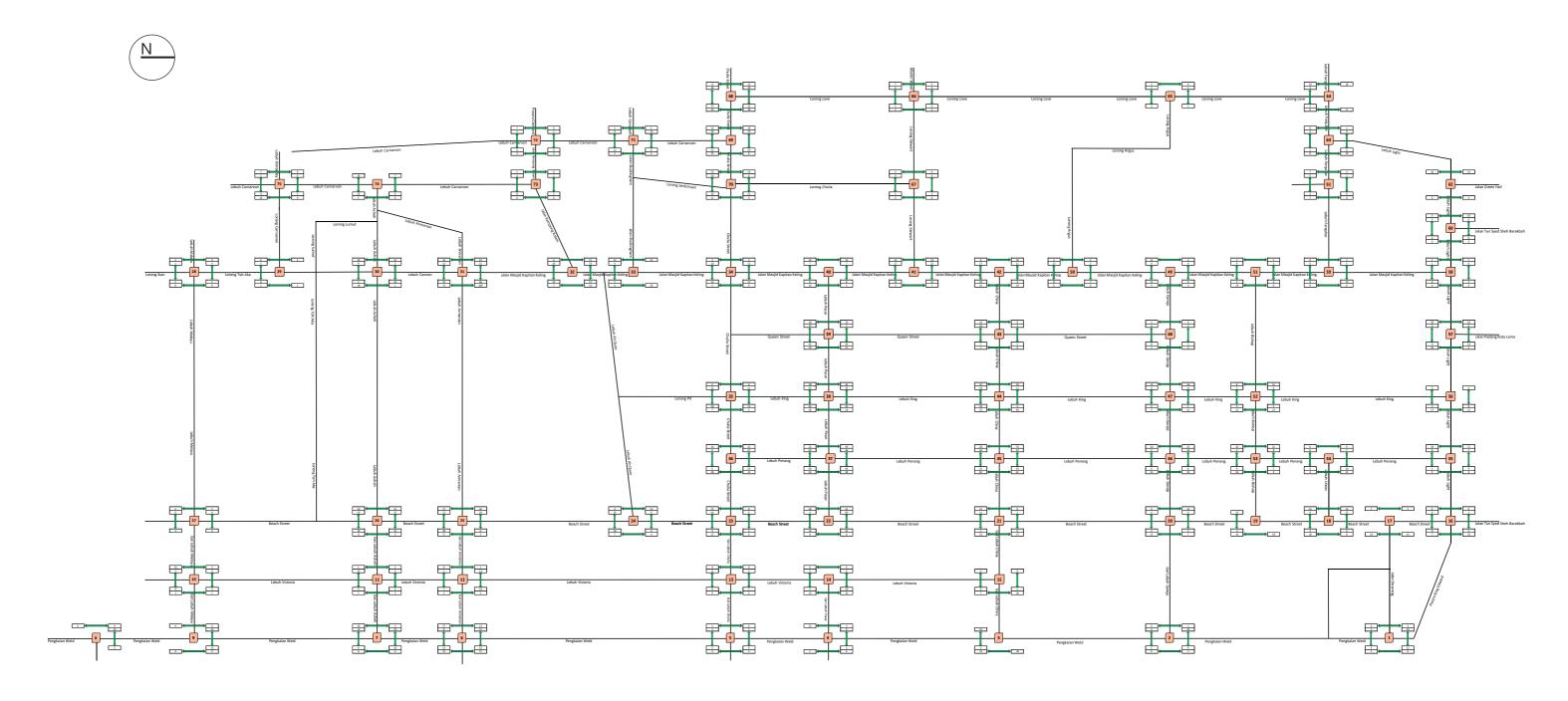


Figure 21: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Full

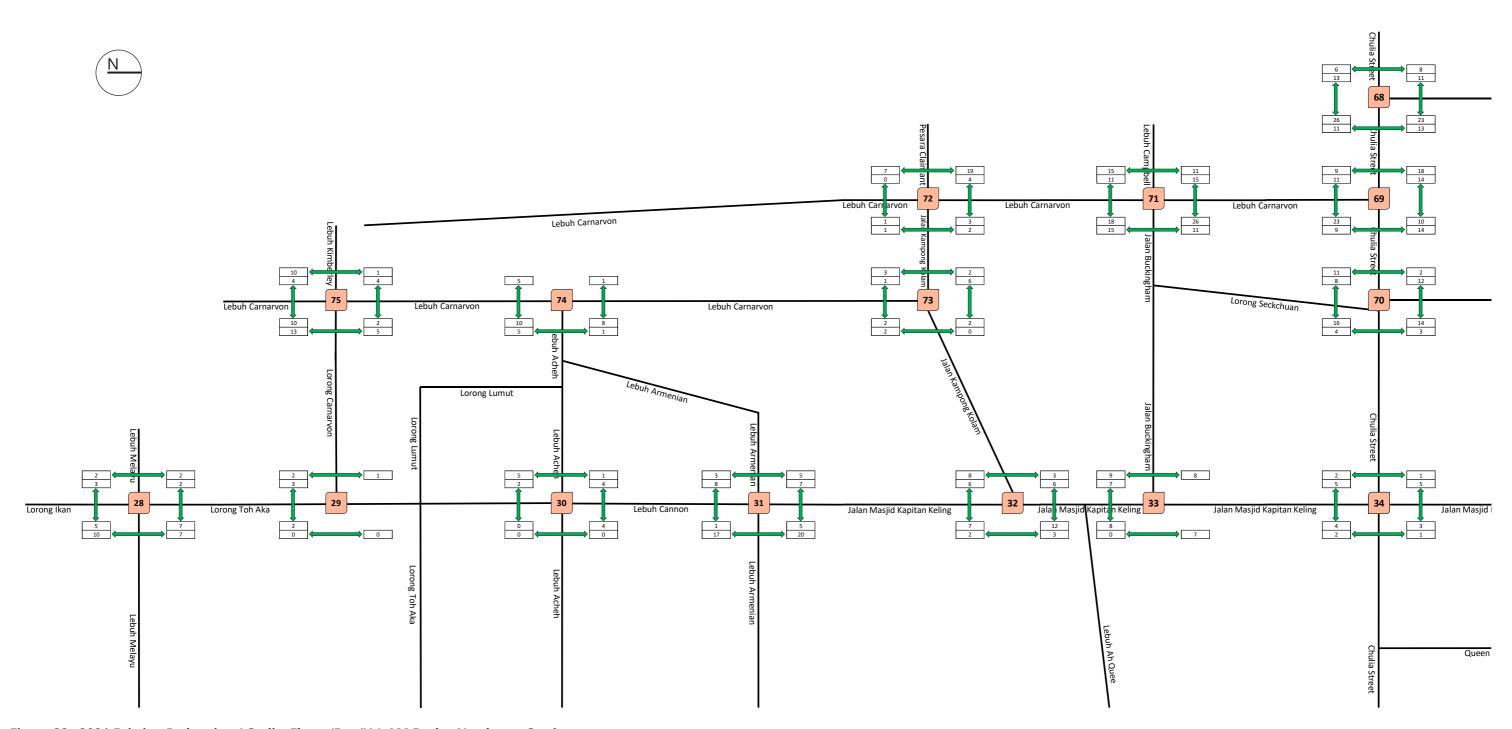


Figure 22: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak - Northwest Section

Figure 23: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Northeast Section

Figure 24: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Southwest Section

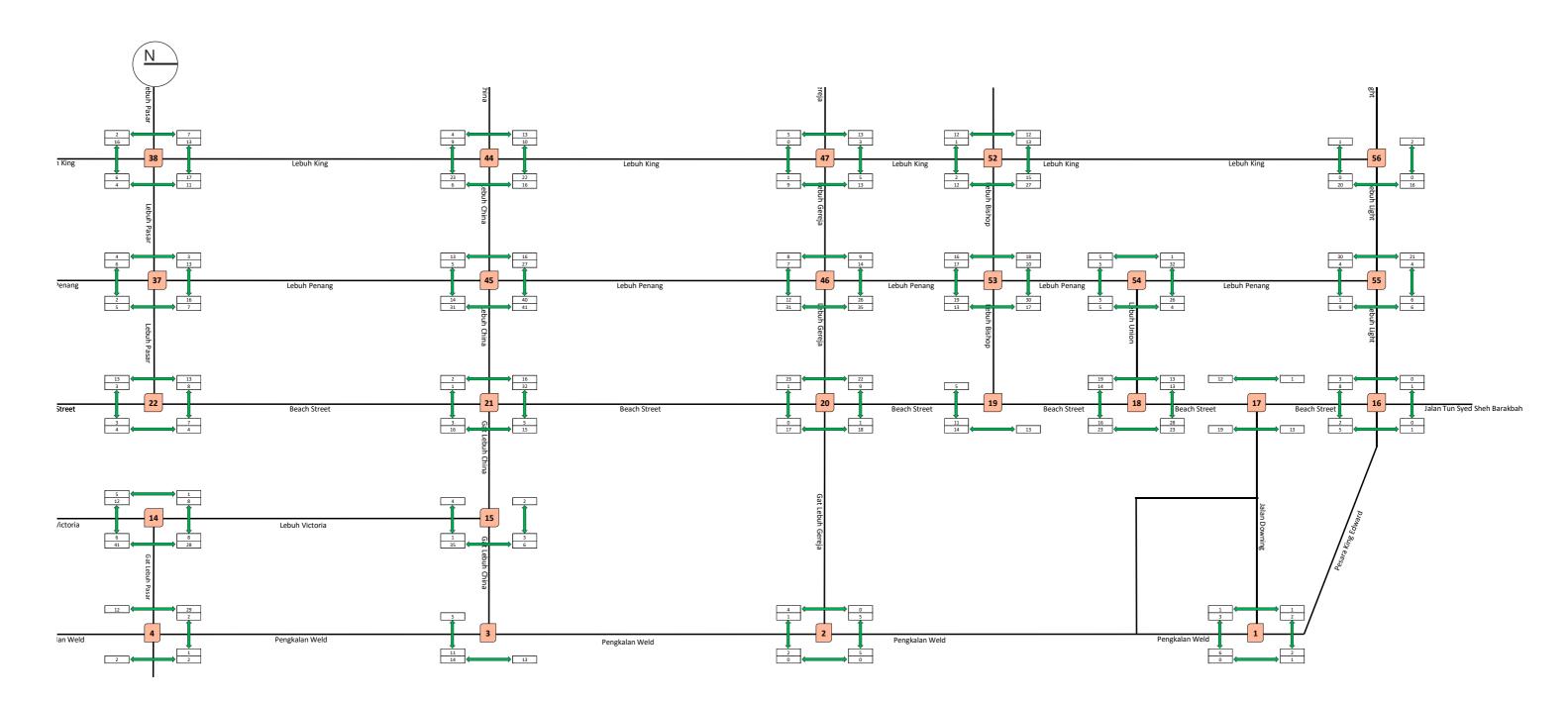


Figure 25: 2021 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) AM Peak – Southeast Section

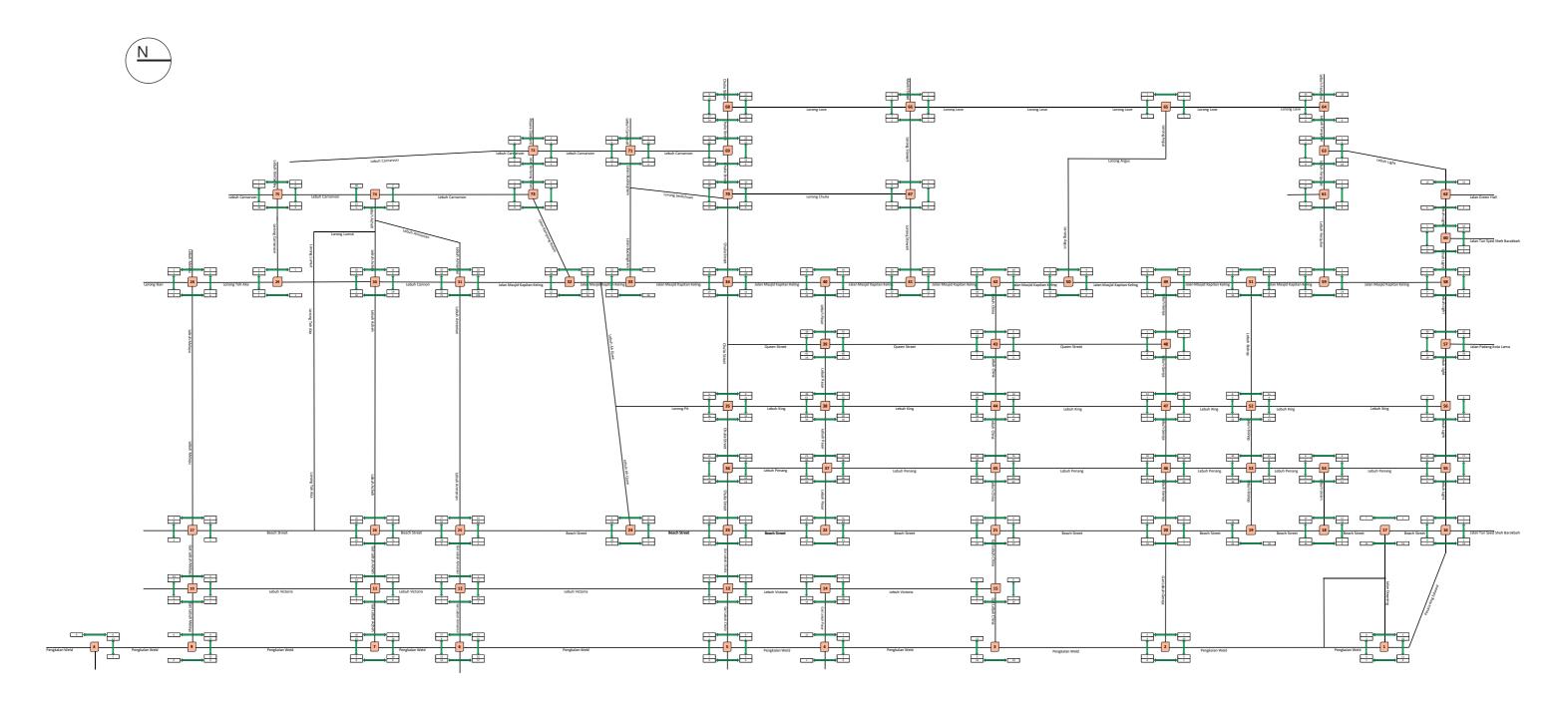
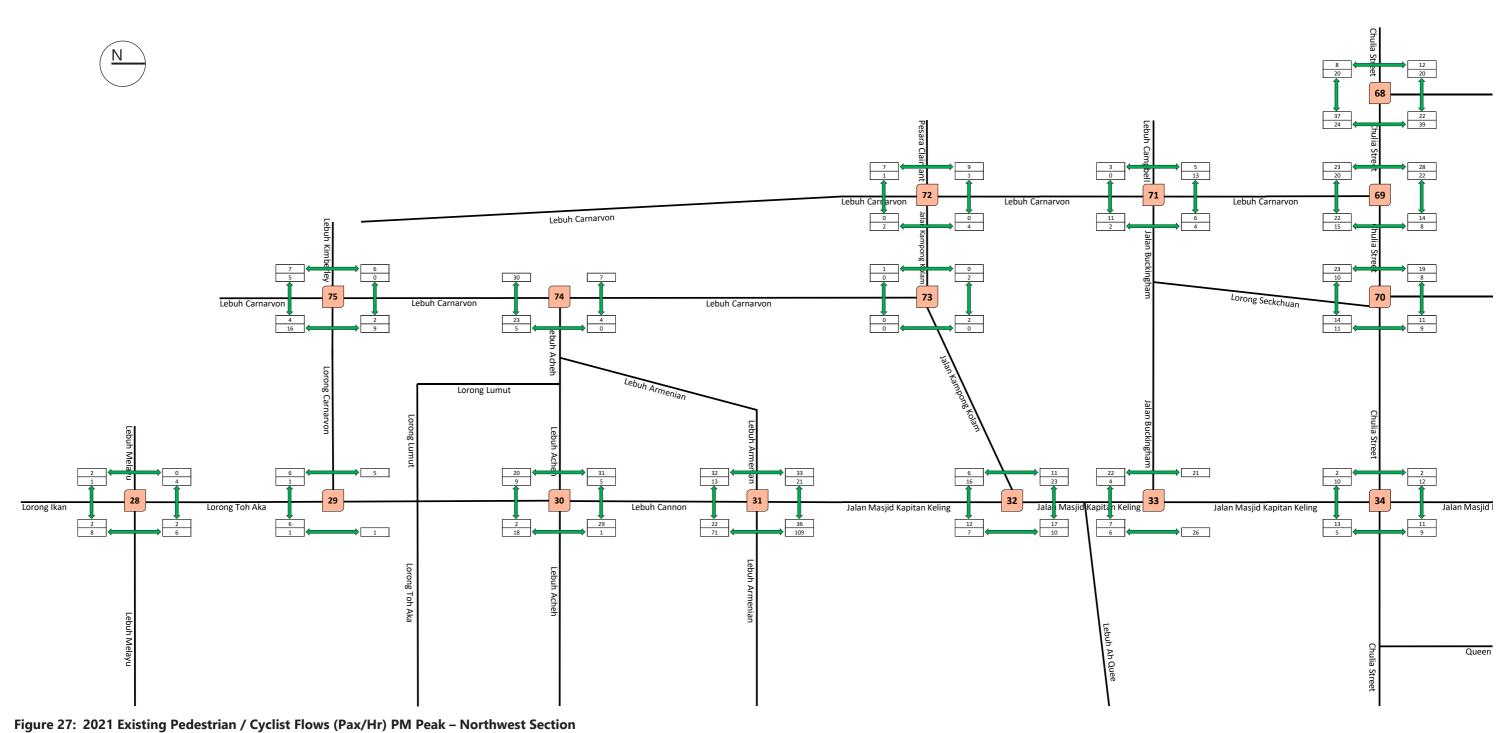


Figure 26: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Full



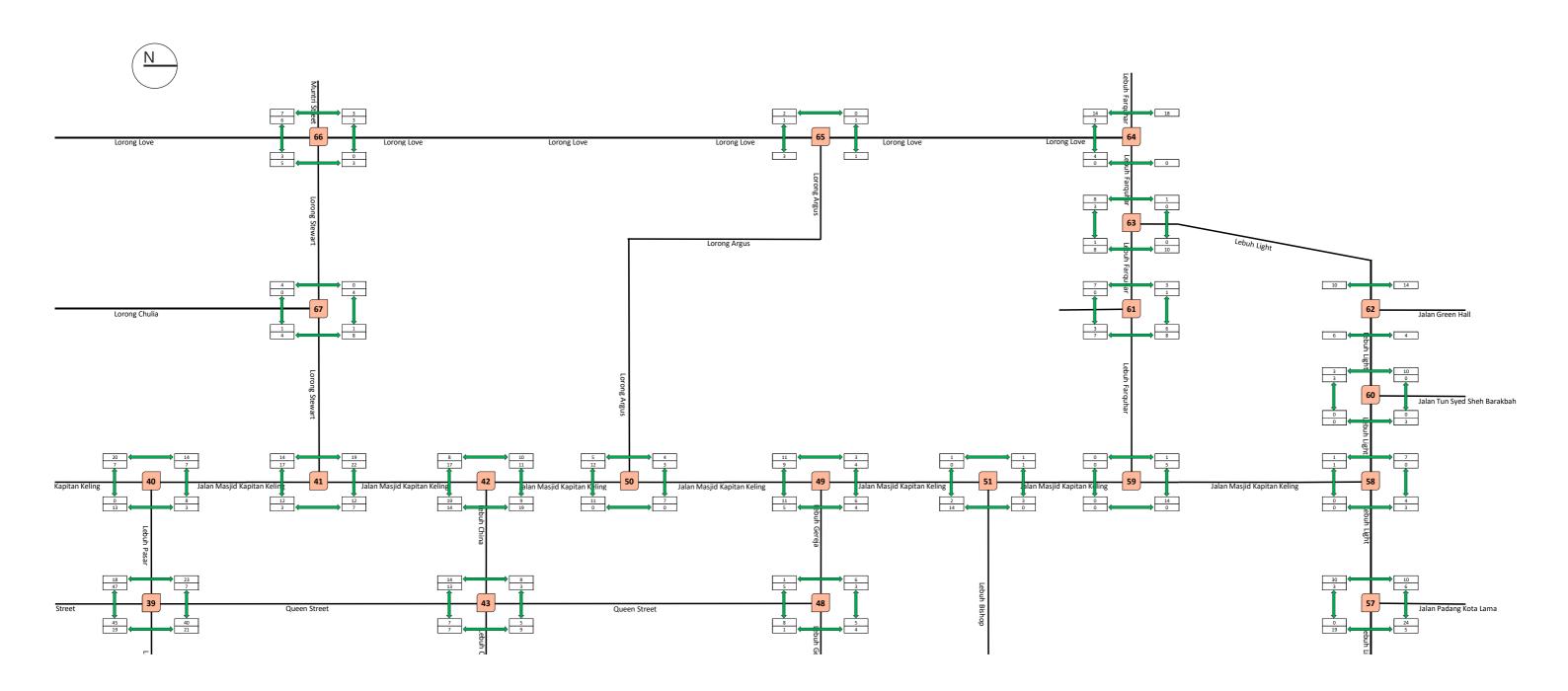


Figure 28: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Northeast Section

Figure 29: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Southwest Section

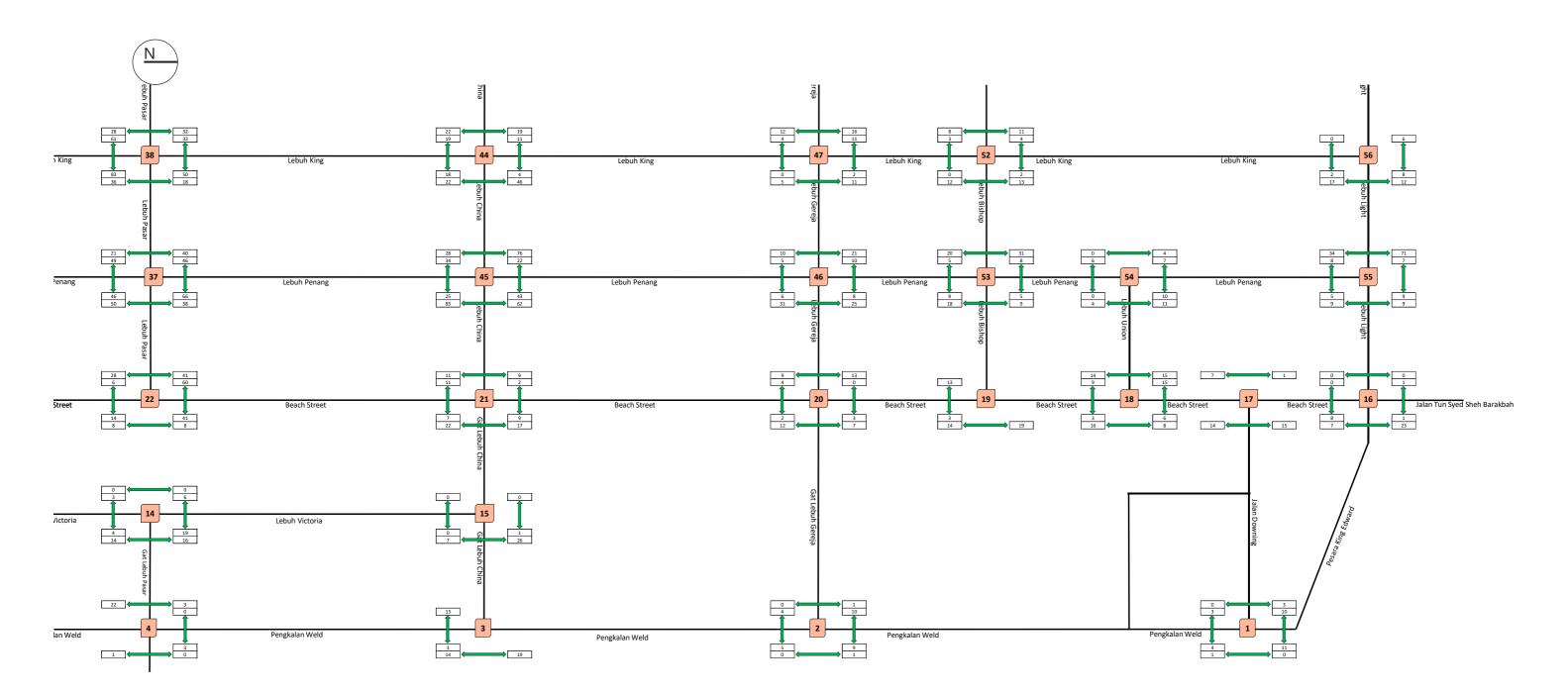


Figure 30: 2021 Existing Pedestrian / Cyclist Flows (Pax/Hr) PM Peak – Southeast Section

3.2.3 JUNCTION LAYOUT AND SIGNAL SURVEYS

As a part of this survey exercise, a full record inventory has been made of existing junction and road layouts, turning movements allowed or banned, traffic signal information, traffic lane configurations, bus stops, internal parking, and public transport facilities, waiting and loading restrictions, and general site layout.

Junction layout and traffic signals after on-site verification with actual situation during the site works from 9 to 11 November 2021 are presented with the following sections.

3.2.3.1 Junction Layout

Junction layout after on-site verification with actual situation during the site works from 9 to 11 November 2021 are presented in Figure 31.



Figure 31: Junction Layout (After On-site Verification)

Further detailed junction layout on individual junction basis is provided in Section 3.3.

3.2.3.2 Junction Signal

Location of signals in the study area are presented in Figure 32.



Figure 32: Junction Signal Location

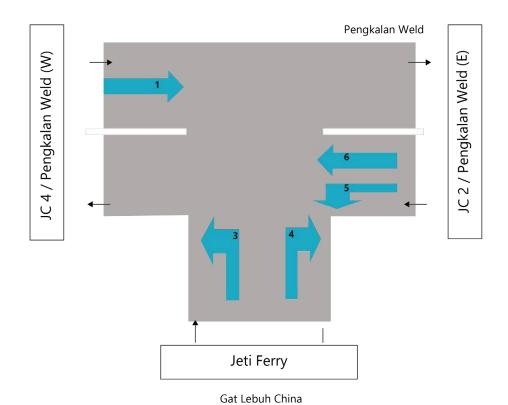
Signal data survey was conducted at 3 locations as indicated in Figure 33.



Figure 33: Surveyed Signalized Junctions

The junction signal for signalized traffic junctions are shown on the next pages.

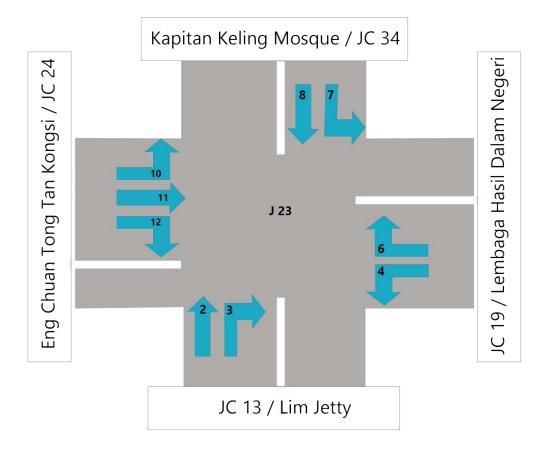
Between Junction 3 & Junction 4



Total Phase All Red Time Phase Movement **Green Time Amber Time** time 3 120 Α 3,4,5 114 3 В 1,2 24 3 3 30 **Total Signal Time** 150

Figure 34: Signal Data for Junction between J3 & J4

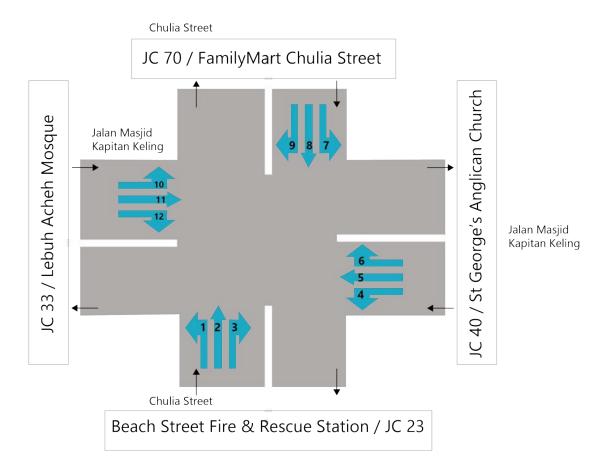
Junction 23:



Phase	Movement	Green Time	Amber Time	All Red Time	Total Phase Time				
А	2,3	51	3	3	57				
В	4,6	32	3	3	38				
С	7,8	45	3	3	51				
D	10,11,12	33	3	3	39				
	Total Signal Time								

Figure 35: Signal Data for Junction 23

Junction 34:



Phase	Movement	Green Time	Amber Time	All Red Time	Total Phase Time			
А	1,2,3	49	3	3	55			
В	4,5,6	32	3	3	38			
С	7,8,9	35	3	3	41			
D	10,11,12	38	3	3	44			
	Total Signal Time							

Figure 36: Signal Data for Junction 34

3.3 **SURVEY OBSERVATIONS**

Survey observations were made at all junctions on-site together with the traffic surveys. The intention of the survey observation is to document the junction layout observed on-site, forming part of the capacity inputs to the modeling exercise.

Detailed survey observations are attached in Appendix A of this report.

QUEUE LENGTH SURVEY 3.4

As proposed in the overall survey plan, queue length survey at existing junctions were conducted to obtain the site condition for model verification and calibration.

Observed queue length for each junction is presented in the table below.

Table 5: Queue Length Survey Results

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Persara King Edward (N)	0	0	Z B
1	В	Jalan Downing (W)	0	0	C A
	С	Pengkalan Weld (S)	0	0	1
	А	Pengkalan Weld (N)	0	0	В
2	В	Gat Lebuh Gereja (W)	0	0	C A
	С	Pengkalan Weld (S)	0	0	2
	А	Pengkalan Weld (N)	0	0	В
3	В	Gat Lebuh China (W)	5	0	C A
	С	Pengkalan Weld (S)	0	0	3
	А	Pengkalan Weld (N)	0	0	В
4	В	Gat Lebuh Pasar (W)	0	0	C
	С	Pengkalan Weld (S)	0	0	4

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Pengkalan Weld (N)	15	55	В
_	В	Gat Lebuh Chulia (W)	5	20	C A
5	С	Pengkalan Weld (S)	0	35	5
	D	Gat Lebuh Chulia (E)	0	0	D
	А	Pengkalan Weld (N)	0	0	В
	В	Gat Lebuh Armenian (W)	0	0	C A
6	С	Pengkalan Weld (S)	0	20	6
	D	Gat Lebuh Armenian (E)	0	0	D
	А	Pengkalan Weld (N)	0	0	В
7	В	Gat Lebuh Acheh (W)	5	25	C A
	С	Pengkalan Weld (S)	0	0	7
	А	Pengkalan Weld (N)	0	0	8
8	В	Pengkalan Weld (S)	0	0	В
	С	Raya Merdeka Hwy (E)	0	10	-
	А	Pengkalan Weld (N)	0	0	В
9	В	Gat Lebuh Melayu (W)	5	25	C A
	С	Pengkalan Weld (S)	0	0	9
	А	Lebuh Victoria (N)	0	0	В
10	В	Gat Lebuh Melayu (W)	0	0	C A
10	С	Lebuh Victoria (S)	0	0	10
	D	Gat Lebuh Melayu (E)	0	0	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Lebuh Victoria (N)	0	0	В
11	В	Gat Lebuh Acheh (W)	0	0	C A
11	С	Lebuh Victoria (S)	0	0	
	D	Gat Lebuh Acheh (E)	0	0	D
	А	Lebuh Victoria (N)	0	0	В
12	В	Gat Lebuh Armenian (W)	0	0	C A
12	С	Lebuh Victoria (S)	0	0	12
	D	Gat Lebuh Armenian (E)	0	0	D
	А	Gat Lebuh Chulia (E)	0	0	В
42	В	Lebuh Victoria (N)	0	0	C A
13	С	Gat Lebuh Chulia (W)	0	0	13
	D	Lebuh Victoria (S)	0	0	D
	А	Lebuh Victoria (N)	0	0	14
14	В	Lebuh Victoria (S)	0	0	В А
	С	Gat Lebuh Pasar (E)	0	0	С
	А	Gat Lebuh China (W)	0	0	А
15	В	Lebuh Victoria (S)	0	0	В 15
	С	Gat Lebuh China (E)	0	0	С
	А	Jalan Tun Syed Sheh Barakbah (N)	0	0	В
	В	Lebuh Light (W)	5	20	C A
16	С	Jalan Tun Syed Sheh Barakbah (S)	5	10	16
	D	Lebuh Light (E)	0	0	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Beach Street (N)	0	0	17
17	В	Beach Street (S)	0	0	В A
	С	Jalan Downing (W)	0	0	С
	А	Beach Street (N)	0	0	В
18	В	Lebuh Union (W)	0	0	C A
	С	Beach Street (S)	0	0	18
	А	Beach Street (N)	0	0	В
19	В	Lebuh Bishop (W)	0	0	C A
	С	Beach Street (S)	0	0	19
	А	Beach Street (N)	0	0	В
20	В	Lebuh Gereja (W)	0	0	C A
20	С	Beach street (S)	0	0	20
	D	Gat Lebuh Gereja (E)	0	0	D
	А	Beach Street (N)	0	0	В
24	В	Lebuh China (W)	0	0	C A
21	С	Beach Street (S)	0	0	21
	D	Gat Lebuh China (E)	0	0	D
	А	Beach Street (N)	0	0	В
22	В	Lebuh Pasar (W)	0	0	C A
	С	Beach Street (S)	0	0	

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Beach Street (N)	10	40	В
22	В	Chulia Street (W)	20	40	C A
23	С	Beach Street (S)	15	25	23
	D	Gat Lebuh Chulia (E)	10	40	D
	А	Beach Street (N)	0	0	В
24	В	Lebuh Ah Quee (W)	5	5	C 24 A
	С	Beach Street (S)	0	0	24
	Α	Beach Street (N)	0	0	В
25	В	Lebuh Armenian (W)	0	0	C A 25
23	С	Beach Street (S)	0	0	
	D	Gat Lebuh Armenian (E)	0	10	D
	А	Beach Street (N)	0	0	В
26	В	Lebuh Acheh (W)	0	0	C A
20	С	Beach Street (S)	0	0	26
	D	Gat Lebuh Acheh (E)	0	0	D
	А	Beach Street (N)	0	0	В
27	В	Lebuh Melayu(W)	0	0	C A
21	С	Beach Street (S)	0	0	
	D	Gat Lebuh Melayu (E)	0	0	D
	А	Lorong Toh Aka (N)	0	0	В
28	В	Lorong Melayu (W)	0	0	C 28 A
20	С	Lorong Ikan (S)	0	0	
	D	Lorong Melayu (E)	0	0	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Lorong Toh Aka (N)	0	0	В
29	В	Lorong Carnarvon (W)	0	0	C A
	С	Lorong Toh Aka (S)	0	0	29
	А	Lebuh Cannon (N)	0	0	В
30	В	Lebuh Acheh (W)	0	0	C A
30	С	Lebuh Cannon (S)	0	0	30
	D	Lebuh Acheh (E)	0	0	D
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
31	В	Lebuh Armenian (W)	0	5	C A A
31	С	Lebuh Cannon (S)	0	0	31
	D	Lebuh Armenian (E)	0	5	, D
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
32	В	Jalan Kampong Kolam (SW)	0	0	C A
	С	Jalan Masjid Kapitan Keling (S)	0	0	32
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
33	В	Jalan Buckingham (W)	0	5	C A
	С	Jalan Masjid Kapitan Keling (S)	0	0	33
	А	Jalan Masjid Kapitan Keling (N)	0	25	В
34	В	Chulia Street (W)	0	50	C A
) J 1	С	Jalan Masjid Kapitan Keling (S)	0	40	
	D	Chulia Street (E)	0	40	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Lebuh King (N)	0	0	В
35	В	Chulia Street (W)	0	0	C 35
33	С	Lorong Pit (S)	0	0	33
	D	Chulia Street (E)	0	0	D
	А	Lebuh Penang (N)	5	10	В
36	В	Chulia Street (W)	0	0	36 A
	С	Chulia Street (E)	0	0	С
	А	Lebuh Penang (N)	0	0	В
27	В	Lebuh Pasar (W)	0	0	C A
37	С	Lebuh Penang (S)	0	0	37
	D	Lebuh Pasar (E)	0	10	D
	Α	Lebuh King (N)	0	0	В
20	В	Lebuh Pasar (W)	0	0	C 38
38	С	Lebuh King (S)	5	10	36
	D	Lebuh Pasar (E)	0	0	D
	Α	Queen Street (N)	0	10	В
20	В	Lebuh Pasar (W)	0	0	C A
39	С	Queen Street (S)	0	0	39
	D	Lebuh Pasar (E)	0	0	D
	А	Jalan Masjid Kapitan Keling (N)	0	0	
40	В	Jalan Masjid Kapitan Keling (S)	0	0	40 A
70	С	Lebuh Pasar (E)	5	3	
	D	Lorong Ikan (S)	0	0	С

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
41	В	Lorong Stewart	5	20	C A
	С	Jalan Masjid Kapitan Keling (S)	0	15	41
	А	Lebuh China (E)	0	0	42
42	В	Jalan Masjid Kapitan Keling (N)	0	0	В А
	С	Jalan Masjid Kapitan Keling (S)	0	15	С
	А	Queen Street (N)	0	0	В
43	В	Lebuh China (W)	0	0	C 43 A
43	С	Queen Street (S)	0	0	
	D	Lebuh China (E)	0	0	D
	А	Lebuh King (N)	0	0	В
44	В	Lebuh China (W)	0	0	C A A
44	С	Lebuh King (S)	0	0	44
	D	Lebuh China (E)	0	0	D
	А	Lebuh Penang (N)	5	10	В
45	В	Lebuh China (W)	0	0	C A
45	С	Lebuh Penang (S)	0	0	45
	D	Lebuh China (E)	0	0	D
	А	Lebuh Penang (N)	0	0	В
	В	Lebuh Gereja (W)	0	0	C A
46	С	Lebuh Penang (S)	0	0	46
	D	Lebuh Gereja (E)	5	10	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Lebuh King (N)	0	0	В
47	В	Lebuh Gereja (W)	0	0	C 47 A
47	С	Lebuh King (S)	5	10	47
	D	Lebuh Gereja (E)	0	0	D
	А	Lebuh Gereja (W)	0	0	А
48	В	Queen Street (S)	0	0	В 48
	С	Lebuh Gereja (E)	0	0	С
	А	Jalan Masjid Kapitan Keling (N)	0	0	49
49	В	Jalan Masjid Kapitan Keling (S)	0	0	В А
	С	Lebuh Gereja (E)	0	35	С
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
50	В	Lorong Argus (W)	0	0	C A
	С	Jalan Masjid Kapitan Keling (S)	0	0	50
	А	Jalan Masjid Kapitan Keling (N)	0	0	
51	В	Jalan Masjid Kapitan Keling (S)	0	5	B A
	С	Lebuh Bishop (E)	0	0	С
	Α	Lebuh King (N)	0	0	В
52	В	Lebuh Bishop (W)	0	0	C 52 A
32	С	Lebuh King (S)	0	0	
	D	Lebuh Bishop (E)	0	0	D
	А	Lebuh Penang (N)	0	5	В
53	В	Lebuh Bishop (W)	0	0	C A
55	С	Lebuh Penang (S)	0	0	
	D	Lebuh Bishop (E)	0	0	D

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Lebuh Penang (N)	0	0	54
54	В	Lebuh Penang (S)	0	0	В А
	С	Lebuh Union (E)	0	5	С
	Α	Lebuh Light (W)	0	0	Α
55	В	Lebuh Penang (S)	0	0	
	С	Lebuh Light (E)	0	0	С
	А	Lebuh Light (W)	0	0	A
56	В	Lebuh King (S)	0	5	B 56
	С	Lebuh Light (E)	0	0	С
	А	Jalan Padang Kota Lama (N)	0	0	В
57	В	Lebuh Light (W)	0	0	57 A
	С	Lebuh Light (E)	0	0	С
	А	Lebuh Light (W)	0	0	A
58	В	Jalan Masjid Kapitan Keling (S)	0	0	
	С	Lebuh Light (E)	0	0	С
	А	Jalan Masjid Kapitan Keling (N)	0	0	В
59	В	Lebuh Farquhar (W)	0	0	C A
	С	Jalan Masjid Kapitan Keling (S)	0	0	39
	А	Jalan Tun Syed Sheh Barakbah (N)	0	10	В
60	В	Lebuh Light (W)	0	0	60 A
	С	Lebuh Light (E)	0	0	С

Junction no.	Approach	Road Name (Direction)	Observed Qeue Length AM (m)	Observed Qeue Length PM (m)	Diagram
	А	Chulia Street (W)	0	0	А
69	В	Lebuh Carnarvon (S)	0	20	В 69
	С	Chulia Street (E)	0	0	С
	А	Lorong Chulia (N)	0	0	В
70	В	Chulia Street (W)	0	0	70 A
	С	Chulia Street (E)	0	0	С
	А	Lebuh Carnarvon (N)	0	0	В
	В	Lebuh Campbell (W)	0	0	C A
71	С	Lebuh Carnarvon (S)	0	0	71
	D	Jalan Buckingham (E)	0	0	D
	А	Lebuh Carnarvon (N)	0	0	В
	В	Pesara Claimant (W)	0	0	C
72	С	Lebuh Carnarvon (S)	0	0	72
	D	Jalan Kampong Kolam (E)	0	0	D
72	А	Jalan Kampong Kolam (W)	0	0	
73	В	Jalan Kampong Kolam (E)	0	0	73
	А	Lebuh Carnarvon (N)	0	0	74
74	В	Lebuh Carnarvon (S)	0	0	В А
	С	Lebuh Acheh (F)	0	0	
	А	Lebuh Carnarvon (N)	0	0	В
75	В	Lebuh Kimberley (W)	0	0	C 75 A
15	С	Lebuh Carnarvon (S)	0	0	
	D	Lorong Carnarvon (E)	0	0	D

63 TRAFFIC SURVEYS

Traffic demand in Penang during the survey period was reduced due to the ongoing Covid-19 pandemic. Residents of Penang as well as tourists have become more cautious around travel during this pandemic period. This has resulted in lower traffic demand on the road-network and in turn has reduced queue lengths often observed in Georgetown.

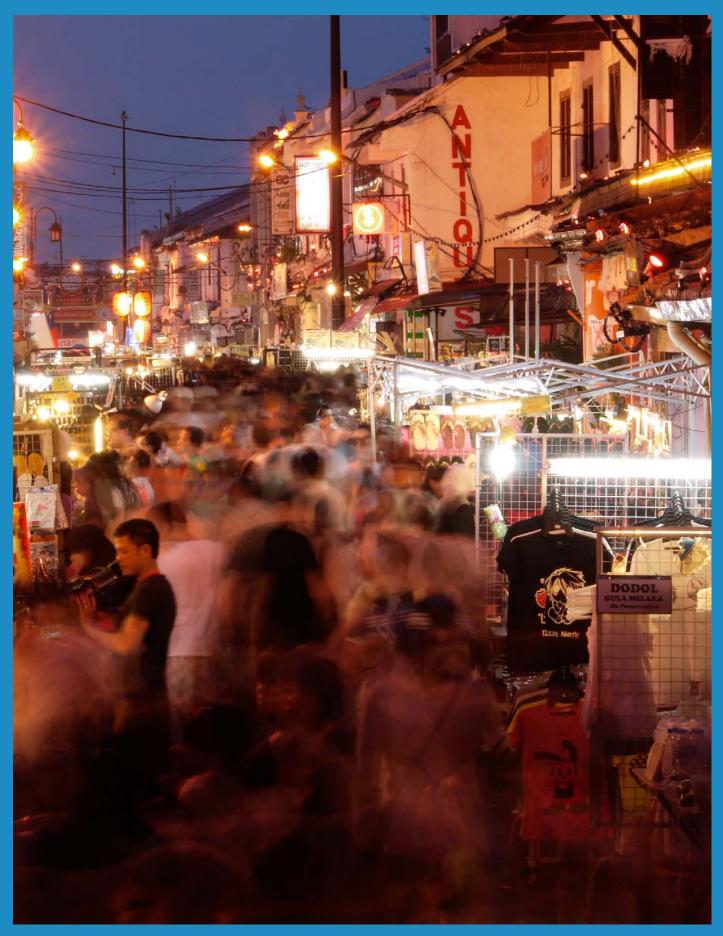


Photo: ADB

PARKING SURVEYS



4.1 SURVEY TASKS

Parking surveys were conducted at both on-street and off-street parking facilities. On-street parking was classified by street and midblock section. Illegal parking was also recorded.

The parking data collected were categorized into the following three tasks:

- 1. On-street parking occupancy survey
- 2. Off-street parking occupancy survey
- 3. Parking dwell time survey

Results of the respective tasks are shown in the sections below.

4.2 SURVEY RESULTS

4.2.1 ON-STREET PARKING OCCUPANCY

On-street parking surveys were conducted between junctions along the road sections highlighted in both yellow and purple in Figure 37.



Figure 37: On-street Parking Occupancy Survey Locations

The entire study area was divided into 4 areas for the survey to be conducted over a three-day period. For each of the surveyed road section between junctions, occupancy data were collected every hour to understand the number of vehicles occupying the parking space at the given time.

Results reported in the table below are showing the maximum and average of the on street parking occupancy. Abbreviations for the content in table is as below:

C - Car;

LGV - Light Good Vehicle; - Heavy Good Vehicle; HGV

В - BUS;

M - Motorcycle;

- Parking on left side of the road while traveling form Junction X to Junction Y; Left Right - Parking on right side of the road while traveling form Junction X to Junction Y.

Table 6: On-street Parking Occupancy Survey Results

Junction 23 to Junction 22				Left					Right			
22		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
AAA Daal	Average	2	0	0	0	1	5	0	0	0	0	
AM Peak	Max	5	0	0	0	2	6	0	0	0	1	
DM D	Average	11	0	0	0	4	10	2	0	0	1	
PM Peak	Max	8	0	0	0	6	10	3	0	0	1	
Junction 22	2 to Junction			Left					Right			
21		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
ANA Deal	Average	4	0	0	0	6	7	0	0	0	5	
AM Peak	Max	4	0	0	0	17	9	0	0	0	10	
DI 4 D	Average	5	0	0	0	17	11	0	0	0	7	
PM Peak	Max	5	0	0	0	18	10	1	0	0	9	
Junction 21	l to Junction			Left					Right			
20		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
AAA Daala	Average	8	0	0	0	11	8	0	0	0	0	
AM Peak	Max	9	0	0	0	21	9	1	0	0	0	
DM Daal	Average	12	0	0	0	26	12	0	0	0	0	
PM Peak	Max	10	0	0	0	27	12	0	0	0	0	
Junction 17	to Junction			Left			Right					
16		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
AAA Daala	Average	0	0	0	0	7	6	0	0	0	4	
AM Peak	Max	0	0	0	0	11	5	0	0	0	10	
DI 4 D	Average	4	0	0	0	5	3	0	0	0	5	
PM Peak	Max	10	0	0	0	9	4	0	0	0	10	
Junction 16	to Junction			Left					Right			
Α		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
AM Deal	Average	2	0	0	0	0	8	0	0	0	0	
AM Peak	Max	6	0	0	0	0	9	0	0	0	1	
DM Deel	Average	1	0	0	0	0	3	0	0	0	0	
PM Peak	Max	1	0	0	0	0	4	0	0	0	0	

Junction 1	to Junction			Left					Right		
17		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Dools	Average	4	0	0	0	17	2	0	0	0	36
AM Peak	Max	4	0	0	0	24	2	0	0	0	63
DIA D	Average	6	0	0	0	13	2	0	0	0	26
PM Peak	Max	5	0	0	0	25	2	0	0	0	50
Junction 5	to Junction			Left					Right		
13		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Devel	Average	4	1	0	0	0	5	0	0	0	2
AM Peak	Max	6	1	0	0	0	11	0	0	0	2
DIAD I	Average	8	0	0	0	0	8	0	0	0	1
PM Peak	Max	9	0	0	0	0	10	0	0	0	1
Junction 13	to Junction			Left					Right		
14		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	6	2	0	0	0	10	0	0	0	3
AM Peak	Max	9	2	0	0	0	11	0	1	0	6
DM Deel	Average	6	1	0	0	0	12	0	0	0	1
PM Peak	Max	7	1	0	0	1	12	0	0	0	1
Junction 14	to Junction			Left					Right		
15		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	1	0	0	0	0	8	0	0	0	0
AM Peak	Max	3	0	0	0	0	7	0	0	0	0
DIAD I	Average	9	0	0	0	0	11	1	0	0	0
PM Peak	Max	10	0	0	0	1	10	1	0	0	0
Junction 21	to Junction			Left					Right		
15		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deals	Average	8	0	0	0	3	0	0	0	0	0
AM Peak	Max	10	0	0	0	8	0	0	0	0	0
DM Deel	Average	9	0	0	0	9	0	0	0	0	0
PM Peak	Max	10	0	0	0	10	0	0	0	0	0
Junction 15	to Junction			Left					Right		
3		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deale	Average	7	0	0	0	0	7	0	0	0	1
AM Peak	Max	10	0	0	0	0	10	0	0	0	4
DM Deel	Average	10	0	0	0	0	9	0	0	0	4
PM Peak	Max	11	0	0	0	1	10	0	0	0	4
Junction 2	to Junction			Left					Right		
20		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deale	Average	22	0	0	0	1	11	0	0	0	0
AM Peak	Max	20	0	0	0	2	14	0	0	0	1
DM D 1	Average	18	0	0	0	0	19	0	0	0	1
PM Peak	Max	17	0	0	0	0	12	0	0	0	1
l	Fad			Left					Right		
Junction 62	: - End	С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 B	Average	11	0	0	0	6	21	1	0	0	9
AM Peak	Max	12	0	0	0	11	27	2	0	0	18
			The second secon								_
PM Peak	Average	17	0	0	0	22	24	2	0	0	15

Junction 58	- Junction			Left					Right		
57		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	13	0	0	0	8	12	0	0	0	0
AM Peak	Max	13	0	0	0	16	17	0	0	0	0
	Average	26	0	0	0	27	31	0	0	0	3
PM Peak	Max	17	1	0	0	25	17	0	0	0	3
			'	Left					Right		
Junction 57	- End	С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	3	1	0	0	7	11	0	0	0	0
AIVI Peak	Max	7	1	0	0	10	28	0	0	0	0
DIA D	Average	10	1	0	0	5	15	0	0	0	0
PM Peak	Max	8	1	0	0	4	16	0	0	0	0
Junction 57	- Junction		'	Left					Right		
56		С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	7	0	0	0	1	0	0	0	0	1
AM Peak	Max	12	0	0	0	2	0	0	0	0	2
	Average	15	0	0	0	0	6	0	0	0	0
PM Peak	Max	12	0	0	0	0	7	0	0	0	0
				Left					Right		
Junction 56	- End	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	25	0	0	0	7	7	0	0	0	9
AM Peak	Max	54	0	0	0	10	13	0	0	0	10
	Average	61	0	0	0	7	16	1	0	0	6
PM Peak	Max	56	0	0	0	10	19	1	0	0	5
Junction 56	- Junction			Left					Right		
16		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	0	0	0	0	2	11	0	0	0	7
AM Peak	Max	1	0	0	0	3	20	1	0	0	8
DIA D	Average	3	0	0	0	1	53	0	0	0	2
PM Peak	Max	5	0	0	0	2	57	1	0	0	5
1	1			Left		•			Right		
Junction 51	- Junction49	С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Dools	Average	2	0	0	0	0	1	0	0	0	0
AM Peak	Max	4	1	0	0	0	4	0	0	0	0
PM Peak	Average	4	0	0	0	4	1	0	0	0	0
PIVI Peak	Max	4	0	0	0	4	2	0	0	0	0
l	- Junction50			Left					Right		•
Junction 49	- Junctionsu	С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Dool-	Average	2	0	0	0	2	3	0	0	0	0
AM Peak	Max	6	0	0	0	3	7	0	0	0	0
DM D!	Average	4	0	0	0	2	4	0	0	0	0
PM Peak	Max	5	0	0	0	2	9	0	0	0	0
lunctic : 50	lum ette i 40			Left					Right		
Junction 50	- Junction42	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANAD	Average	2	0	0	0	7	1	0	0	0	6
AM Peak	Max	2	0	0	0	11	2	0	0	0	6
DM D	Average	0	0	0	0	4	3	0	0	0	8
PM Peak	Max	0	0	0	0	8	4	0	0	0	8

lunction 24	l lungtion 70			Left					Right		
Junction 34	-Junction70	C	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	2	0	0	0	3	7	0	0	0	0
AIVI Peak	Max	4	0	0	0	6	13	0	0	0	1
DIA D	Average	9	0	0	0	8	13	0	0	0	2
PM Peak	Max	12	0	0	0	12	15	1	0	0	2
				Left					Right		
Lorong Ste	ward	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Devel	Average	1	0	0	0	0	2	0	0	0	1
AM Peak	Max	2	0	0	0	0	2	0	0	0	2
D14 D	Average	9	1	0	0	0	3	0	0	0	1
PM Peak	Max	10	2	0	0	0	3	0	0	0	2
				Left					Right		
Junction 50) - End	С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D	Average	4	0	0	0	0	0	0	0	0	0
AM Peak	Max	8	0	0	0	0	0	0	0	0	0
D14.5 :	Average	7	0	0	0	0	0	0	0	0	0
PM Peak	Max	9	0	0	0	0	0	0	0	0	0
	_			Left					Right		
A to B Loro	ng Muda	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	0	0	0	0	5	3	0	0	0	0
AM Peak	Max	0	0	0	0	7	3	0	0	0	0
	Average	0	0	0	0	2	5	0	0	0	0
PM Peak	Max	0	0	0	0	4	8	0	0	0	0
a. =				Left					Right		
C to D		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	1	0	0	0	6	14	0	0	0	1
AM Peak	Max	3	0	0	0	6	17	0	0	0	1
D14 D	Average	9	0	0	0	2	22	0	0	0	0
PM Peak	Max	14	1	0	0	2	21	0	0	0	0
				Left					Right		
Lebuh Klan	g	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	3	0	0	0	4	6	0	0	0	1
AM Peak	Max	4	0	0	0	4	5	0	0	0	1
DM Daal	Average	5	0	0	0	4	6	0	0	0	1
PM Peak	Max	6	0	0	0	5	6	0	0	0	3
loon ations C7				Left					Right		
Junction 67	'-Junction70	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	6	1	0	0	4	5	0	0	0	1
AM Peak	Max	9	1	0	0	4	7	0	0	0	3
DM D 1	Average	7	0	0	0	2	8	0	0	0	2
PM Peak	Max	7	0	0	0	3	7	0	0	0	3
l				Left					Right		
Junction 68	3 -Junction66	С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D . '	Average	0	0	0	0	2	16	0	0	0	13
AM Peak	Max	1	0	0	0	2	18	0	0	0	8
	Average	3	0	0	0	5	15	0	0	0	16
PM Peak	, werage										

lunction 66	-Junction65			Left					Right		
Junction 66	-Junctiones	C	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	9	0	0	0	5	0	0	0	0	1
AIVI FEAK	Max	13	0	0	0	5	0	0	0	0	1
PM Peak	Average	11	0	0	0	5	0	0	0	0	1
PIVI PEAK	Max	11	0	0	0	6	0	0	0	0	1
Junction 65	- Fra al			Left					Right		
Junction 65	- ENG	C	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	4	0	0	0	0	0	1	0	0	1
AIVI FEAK	Max	3	0	0	0	0	0	1	0	0	1
PM Peak	Average	4	0	0	0	0	0	0	0	0	1
PIVI Peak	Max	4	0	0	0	1	0	0	0	0	2
eti CF	·			Left					Right		
Junction 65	-Junction64	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deale	Average	7	0	0	0	1	0	0	0	0	0
AM Peak	Max	12	0	0	0	1	0	0	0	0	0
DM Deals	Average	5	0	0	0	0	0	0	0	0	0
PM Peak	Max	5	0	0	0	0	0	0	0	0	0
Junction 59	to			Left					Right		•
Junction52		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deele	Average	5	0	0	0	1	5	0	0	0	7
AM Peak	Max	11	0	0	0	1	8	0	0	0	10
DM Deel	Average	13	0	0	0	8	13	0	0	0	11
PM Peak	Max	11	0	0	0	9	18	0	0	0	13
Junction 52	to Junction			Left					Right		
53		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Dools	Average	6	0	0	0	8	8	0	0	0	1
AM Peak	Max	6	1	0	0	10	8	0	0	0	3
DM Deed	Average	10	0	0	0	8	9	0	0	0	4
PM Peak	Max	8	0	0	0	9	8	0	0	0	5
Junction 53	to Junction			Left					Right		
19		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Devel	Average	5	0	0	0	2	16	0	0	0	7
AM Peak	Max	5	0	0	0	5	23	0	0	0	8
DM Deel	Average	15	1	0	0	11	23	0	0	0	18
PM Peak	Max	10	1	0	0	12	21	0	0	0	21
Junction 18	to Junction			Left					Right		
54		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Devel	Average	10	0	0	0	11	15	0	0	0	10
AM Peak	Max	10	0	0	0	20	20	0	0	0	13
DM D!	Average	18	0	0	0	9	19	0	0	0	7
PM Peak	Max	13	0	0	0	17	19	0	0	0	13
Junction 55	to Junction			Left					Right		
54		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	3	0	0	0	6	5	0	0	0	0
AM Peak	Max	3	0	0	0	8	7	0	0	0	0
							_		_		_
PM Peak	Average	4	0	0	0	8	8	0	0	0	0

Junction 54	to Junction			Left					Right		
53		С	LGV	HGV	В	М	С	LGV	HGV	В	М
4445	Average	4	0	0	0	5	7	1	0	0	1
AM Peak	Max	5	1	0	0	8	10	1	0	0	2
	Average	5	0	0	0	7	9	0	0	0	1
PM Peak	Max	6	0	0	0	7	11	0	0	0	2
Junction 53	to Junction			Left					Right		
46	to Juneuon	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	6	0	0	0	2	4	0	0	0	5
AM Peak	Max	5	0	0	0	3	8	0	0	0	5
	Average	7	0	0	0	1	11	0	0	0	4
PM Peak	Max	6	0	0	0	1	11	0	0	0	5
lunction 46	to Junction			Left					Right		
45	to surretion	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	8	0	0	0	4	10	0	0	0	3
AM Peak	Max	12	0	0	0	4	16	0	0	0	4
	Average	13	0	0	0	8	19	0	0	0	4
PM Peak	Max	10	0	0	0	6	18	0	0	0	3
lunction 45	toJunction			Left					Right		
37	tosunction	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	2	0	0	0	1	6	0	0	0	2
AM Peak	Max	3	0	0	0	2	10	0	0	0	2
	Average	6	1	0	0	4	11	0	0	0	8
PM Peak	Max	5	1	0	0	5	11	0	0	0	7
lunction 27	' to Junction	3		Left	- U		''		Right	- U	,
36	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	5	0	0	0	2	6	0	0	0	5
AM Peak	Max	7	1	0	0	4	12	0	0	0	9
	Average	16	1	0	0	5	15	0	0	0	8
PM Peak	Max	11	2	0	0	5	12	0	0	0	12
lunction 22	to Junction		_	Left			. =		Right	Ţ.	.=
22	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	2	0	0	0	3	6	0	0	0	1
AM Peak	Max	5	0	0	0	3	9	0	0	0	3
	Average	11	0	0	0	6	9	2	0	0	1
PM Peak	Max	9	0	0	0	7	10	4	0	0	4
lunction 22	to Junction			Left			. 0		Right		
21	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	5	0	0	0	8	7	0	0	0	6
AM Peak	Max	4	0	0	0	21	9	0	0	0	12
	Average	4	0	0	0	20	9	1	0	0	10
PM Peak	Max	4	0	0	0	21	9	2	0	0	15
lunction 21	to Junction	·		Left	, ,		J		Right	, ,	
20	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	7	0	0	0	19	8	0	0	0	0
AM Peak	Max	8	0	0	0	27	10	1	0	0	0
	Average	11	0	0	0	30	12	0	0	0	1
PM Peak	Max	8	0	0	0	26	13	0	0	0	2
	IVION	U		U	U	20	۱۵	U	U	U	۷

Junction 20) to Junction			Left					Right		
46		С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D	Average	14	3	0	0	6	9	4	1	0	5
AM Peak	Max	15	2	0	0	9	14	5	2	0	8
DM D	Average	20	2	0	0	7	15	3	0	0	11
PM Peak	Max	17	2	0	0	5	15	4	0	0	10
Junction 46	to Junction			Left					Right		
47		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	6	0	0	0	0	5	0	0	0	0
AM Peak	Max	8	0	0	0	0	10	0	0	0	0
DM Daal	Average	8	0	0	0	0	9	0	0	0	0
PM Peak	Max	8	0	0	0	0	9	0	0	0	0
Junction 47	to Junction			Left					Right		
48		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	4	0	0	0	2	7	0	0	0	1
AM Peak	Max	7	0	0	0	3	12	0	0	0	2
DM Deed	Average	6	0	0	0	2	12	1	0	0	0
PM Peak	Max	6	0	0	0	3	12	1	0	0	1
Junction 48	to Junction			Left					Right		
49		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	2	0	0	0	1	2	0	0	0	0
AM Peak	Max	3	0	0	0	4	4	0	0	0	0
DM Deed	Average	3	0	0	0	2	3	0	0	0	0
PM Peak	Max	3	0	0	0	2	4	0	0	0	0
Junction 42	to Junction			Left					Right		
43		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	1	0	0	0	1	1	0	0	0	0
Alvi Feak	Max	2	0	0	0	1	4	0	0	0	0
PM Peak	Average	3	0	0	0	1	4	0	0	0	0
FIVI FEAK	Max	3	0	0	0	2	4	1	0	0	0
Junction 43	to Junction			Left					Right		
44		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	2	0	0	0	0	1	0	0	0	1
AWITCUK	Max	5	0	0	0	1	2	0	0	0	2
PM Peak	Average	7	0	0	0	4	8	0	0	0	2
1 WIT CUK	Max	5	0	0	0	3	6	0	0	0	2
	to Junction			Left					Right		
45		С	LGV	HGV	В	M	С	LGV	HGV	В	М
AM Peak	Average	5	0	0	0	5	5	0	0	0	0
, avi i cak	Max	5	0	0	0	6	8	0	0	0	0
PM Peak	Average	9	0	0	0	4	8	0	0	0	3
. IVI I CUR	Max	6	0	0	0	6	8	1	0	0	4
	to Junction			Left					Right		
21		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	8	0	0	0	4	10	2	0	0	4
·······································	Max	9	1	0	0	7	14	2	0	0	11
PM Peak	Average	15	0	0	0	8	15	1	0	0	6
Cak	Max	9	0	0	0	9	16	1	0	0	9

Junction 22	to Junction			Left					Right		
37		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De el	Average	12	0	0	0	3	4	1	0	0	0
AM Peak	Max	21	0	0	0	5	5	1	0	0	1
D1 4 D 1	Average	21	0	0	0	10	12	1	0	0	3
PM Peak	Max	18	0	0	0	7	10	1	0	0	4
Junction 35	to Junction			Left					Right		
38		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	5	0	0	0	2	8	0	0	0	3
AIVI PEAK	Max	5	0	0	0	4	11	1	0	0	4
DM Deel	Average	19	0	0	0	9	15	0	0	0	8
PM Peak	Max	15	0	0	0	12	13	0	0	0	7
Junction 38	to Junction			Left		•			Right		
44		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deals	Average	2	1	0	0	0	5	0	0	0	4
AM Peak	Max	6	1	0	0	0	9	0	0	0	7
DIA D	Average	10	2	0	0	2	9	0	0	0	6
PM Peak	Max	6	2	0	0	2	9	0	0	0	6
Junction 44	to Junction			Left					Right		
47		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De al	Average	7	1	0	0	3	8	0	0	0	0
AM Peak	Max	14	2	0	0	4	13	0	0	0	1
DIA D	Average	19	0	0	0	5	15	0	0	0	4
PM Peak	Max	17	0	0	0	7	14	0	0	0	5
Junction 47	to Junction			Left		•			Right		
52		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	10	0	0	0	1	6	0	0	0	2
Alvi Feak	Max	14	0	0	0	2	7	0	0	0	3
PM Peak	Average	16	0	0	0	0	8	0	0	0	0
PIVI Peak	Max	14	0	0	0	0	8	0	0	0	0
Junction 52	to Junction			Left					Right		•
56		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	24	0	0	0	1	18	1	0	0	10
AIVI Peak	Max	27	0	0	0	2	25	1	0	0	17
PM Peak	Average	26	0	0	0	2	23	1	0	0	10
PIVI PEAK	Max	28	0	0	0	4	28	1	0	0	17
Junction 48	to Junction			Left					Right		
43		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	4	0	0	0	2	15	0	0	0	0
AIVI PEAK	Max	8	0	0	0	2	25	0	0	0	1
DM Dools	Average	8	0	0	0	4	21	0	0	0	2
PM Peak	Max	10	0	0	0	5	25	0	0	0	3
Junction 43	to Junction			Left					Right		
39		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA D !	Average	2	0	0	0	4	7	0	0	0	3
AM Peak	Max	3	0	0	0	4	11	0	0	0	4
DM Daal	Average	5	0	0	0	3	15	0	0	0	7
PM Peak	Max	6	0	0	0	3	12	0	0	0	8

Junction 34	to Junction			Left					Right		
35		С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 B	Average	0	0	0	0	2	4	0	0	0	0
AM Peak	Max	0	0	0	0	3	7	0	0	0	0
	Average	1	0	0	0	6	7	0	0	0	2
PM Peak	Max	2	1	0	0	7	8	0	0	0	2
Junction 35	to Junction			Left					Right		
23		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AMA Deels	Average	6	0	0	0	1	8	1	0	0	2
AM Peak	Max	11	0	0	0	4	9	2	0	0	5
DI I D	Average	9	0	0	0	5	14	1	0	0	6
PM Peak	Max	11	0	0	0	6	16	1	0	0	7
				Left					Right		
Junction 39	to End	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA D	Average	7	0	0	0	4	9	0	0	0	13
AM Peak	Max	6	0	0	0	5	11	0	0	0	10
D14.5 '	Average	9	0	0	0	10	16	0	0	0	9
PM Peak	Max	7	0	0	0	12	12	0	0	0	8
Junction 71	to Junction			Left					Right		
С		С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D	Average	1	0	0	0	1	7	0	0	0	6
AM Peak	Max	1	0	0	0	2	10	0	0	0	6
DIA D	Average	0	0	0	0	0	5	0	0	0	2
PM Peak	Max	0	0	0	0	0	6	0	0	0	4
Junction C	to Junction			Left					Right		
32		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	1	0	0	0	0	9	0	0	0	0
AIVI PEAK	Max	2	0	0	0	0	13	0	0	0	0
DM Deel	Average	7	0	0	0	0	15	0	0	0	1
PM Peak	Max	8	0	0	0	0	15	0	0	0	2
looration D	4. l			Left				•	Right		•
Junction D	to Junction C	С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	4	0	0	0	0	6	1	0	0	0
AIVI Peak	Max	5	0	0	0	0	6	1	0	0	0
DM Dools	Average	5	0	0	0	0	6	0	0	0	0
PM Peak	Max	5	0	0	0	0	6	0	0	0	0
Junction 31	to Junction			Left					Right		
73		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	0	0	0	0	0	3	0	0	0	2
AIVI PEAK	Max	0	0	0	0	0	5	0	0	0	4
DM Dools	Average	0	0	0	0	0	7	0	0	0	2
PM Peak	Max	1	0	0	0	0	9	0	0	0	3
Junction 73	to Junction			Left					Right		
74		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AMA D I	Average	5	0	0	0	5	0	0	0	0	1
AM Peak	Max	6	0	0	0	6	0	0	0	0	3
DM Daal	Average	5	0	0	0	4	0	0	0	0	0
PM Peak	Max	4	0	0	0	4	1	0	0	0	0

Junction 74	to Junction			Left					Right		
75		С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D	Average	11	0	0	0	2	2	0	0	0	0
AM Peak	Max	9	0	0	0	7	5	1	0	0	0
DM D	Average	8	0	0	0	8	0	0	0	0	0
PM Peak	Max	7	0	0	0	6	0	0	0	0	0
Junction 75	to Junction			Left					Right		
29		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA De al	Average	18	3	0	0	6	4	1	0	0	2
AM Peak	Max	18	3	0	0	5	5	1	0	0	4
DM D	Average	20	2	0	0	7	6	1	0	0	3
PM Peak	Max	17	2	0	0	4	5	2	0	0	4
Junction 29	to Junction			Left					Right		
28		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	9	0	0	0	0	0	0	0	0	0
AM Peak	Max	11	0	0	0	1	0	0	0	0	0
DM Deed	Average	9	0	0	0	0	0	0	0	0	0
PM Peak	Max	10	0	0	0	1	0	0	0	0	0
Junction 27	to Junction			Left		•			Right		
28		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	1	0	0	0	0	4	0	0	0	3
AM Peak	Max	1	0	0	0	1	9	0	0	0	5
DM Deed	Average	4	0	0	0	1	6	1	0	0	1
PM Peak	Max	3	0	0	0	1	7	1	0	0	2
Junction 27	to Junction			Left				•	Right		•
26		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	21	2	0	0	4	8	2	0	0	2
AIVITEAN	Max	22	3	0	0	5	8	2	0	0	4
PM Peak	Average	20	2	0	0	3	11	2	0	0	13
1 IVI I Cak	Max	20	2	0	0	6	11	2	0	0	20
	to Junction			Left					Right		
25		С	LGV	HGV	В	M	С	LGV	HGV	В	М
AM Peak	Average	9	0	0	0	5	5	1	0	0	1
7 HVI I COIN	Max	14	0	0	0	4	13	2	0	0	3
PM Peak	Average	18	0	0	0	2	13	2	0	0	1
1 W Cak	Max	19	0	0	0	3	15	2	0	0	2
	to Junction			Left					Right		
24		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AM Peak	Average	4	1	0	0	2	1	2	0	0	1
Cuit	Max	8	1	0	0	5	2	2	0	0	2
PM Peak	Average	9	2	0	0	2	8	2	0	0	5
·······································	Max	11	3	0	0	5	7	3	0	0	5
	to Junction			Left					Right		
23		С	LGV	HGV	В	M	С	LGV	HGV	В	М
AM Peak	Average	3	0	0	0	0	5	0	0	0	0
	Max	5	1	0	0	0	7	0	0	0	0
PM Peak	Average	5	0	0	0	0	5	0	0	0	0
	Max	5	0	0	0	0	7	1	0	0	0

Junction 13	to Junction			Left					Right			
12		С	LGV	HGV	В	М	С	LGV	HGV	В	М	
414 D. I	Average	9	1	0	0	0	2	0	0	0	3	
AM Peak	Max	16	1	0	0	1	2	0	0	0	4	
	Average	15	1	0	0	0	8	0	0	0	4	
PM Peak	Max	16	1	0	0	0	9	0	0	0	4	
Junction 12	to Junction			Left					Right			
11	to building.	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	1	0	0	0	0	3	0	0	0	1	
AM Peak	Max	2	0	0	0	1	5	0	0	0	2	
	Average	3	0	0	0	0	8	1	0	0	4	
PM Peak	Max	3	1	0	0	1	8	1	0	0	7	
lunction 11	to Junction			Left					Right			
10	to Junetion	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	8	1	0	0	2	22	2	0	0	3	
AM Peak	Max	10	1	0	0	3	22	2	0	0	3	
	Average	12	2	0	0	1	25	2	0	0	7	
PM Peak	Max	10	1	1	0	3	24	2	0	0	6	
lunction 10	to Junction			Left					Right			
27	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	0	0	0	0	0	6	0	0	0	1	
AM Peak	Max	0	1	0	0	0	6	0	0	0	2	
	Average	1	0	0	0	0	7	0	0	0	1	
PM Peak	Max	3	0	0	0	0	7	1	0	0	2	
Junction 6 t		3		Left	Ü		,		Right	Ü	_	
12	to Junction	С	LGV	HGV	В	М	C LGV HGV B M					
	Average	9	0	0	0	3	2	1	0	0	2	
AM Peak	Max	9	0	0	0	2	2	1	0	0	4	
	Average	10	0	0	0	6	7	1	0	0	0	
PM Peak	Max	10	0	0	0	5	9	1	0	0	0	
lunction 12	to Junction	.0		Left	-				Right	Ţ,		
25	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	4	1	0	0	1	6	0	0	0	0	
AM Peak	Max	5	1	0	0	1	10	0	0	0	0	
	Average	11	1	0	0	3	15	0	0	0	0	
PM Peak	Max	11	1	0	0	4	15	0	0	0	1	
lunction 2E	to Junction			Left	Ü		13		Right	Ŭ		
E	to Junction	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	0	0	0	0	0	1	0	0	0	0	
AM Peak	Max	1	0	0	0	1	3	0	0	0	1	
	Average	3	0	0	0	0	0	0	0	0	0	
PM Peak	Max	4	0	0	0	0	1	0	0	0	1	
	IVIUA	7		Left	<u> </u>		'		Right	U	<u> </u>	
Junction E t	to Junction F	С	LGV	HGV	В	М	С	LGV	HGV	В	М	
	Average	1	0	0	0	1	1	0	0	0	0	
AM Peak	Max	2	0	0	0	2	2	0	0	0	0	
		4	0	0	0	1	5	0	0	0	1	
PM Peak	Average		0				6			0		
	Max	4	U	0	0	2	р	0	0	U	2	

Junction E t	to Junction			Left					Right		
31		С	LGV	HGV	В	М	С	LGV	HGV	В	М
4	Average	1	0	0	0	1	4	0	0	0	0
AM Peak	Max	2	0	0	0	4	5	0	0	0	0
	Average	4	0	0	0	0	4	0	0	0	1
PM Peak	Max	4	0	0	0	0	4	0	0	0	1
Junction 31	to Junction			Left					Right		
30		С	LGV	HGV	В	М	С	LGV	HGV	В	М
414 D. I	Average	4	0	0	0	1	2	0	0	0	0
AM Peak	Max	5	0	0	0	1	2	0	0	0	0
	Average	12	0	0	0	1	3	0	0	0	0
PM Peak	Max	12	0	0	0	2	2	0	0	0	0
Junction 30	to Junction			Left					Right		
G		С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	0	0	0	0	0	2	0	0	0	0
AM Peak	Max	0	0	0	0	0	2	0	0	0	0
D145 :	Average	0	0	0	0	0	5	0	0	0	1
PM Peak	Max	0	0	0	0	1	7	0	0	0	1
Junction 74	to Junction			Left					Right		
31		С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	11	0	0	0	0	0	0	0	0	0
AM Peak	Max	11	0	0	0	0	0	0	0	0	0
	Average	11	0	0	0	0	3	0	0	0	1
PM Peak	Max	12	0	0	0	0	3	0	0	GV	1
Junction 30	to Junction			Left					Right		
PM Peak Average Max Junction 30 to Junctio		С	LGV	HGV	В	М	С	LGV	HGV	В	М
4445	Average	9	0	0	0	8	5	0	0	0	0
AM Peak	Max	14	0	0	0	10	6	0	0	0	0
	Average	17	0	0	0	6	5	0	0	0	2
PM Peak	Max	20	0	0	0	7	6	0	0	0	4
Junction 26	to Junction			Left					Right		
11		С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	1	0	0	0	1	7	0	0	0	1
AM Peak	Max	2	0	0	0	1	8	0	0	0	1
	Average	1	0	0	0	0	9	0	0	0	3
PM Peak	Max	1	0	0	0	0	9	0	0	0	5
Junction 11	to Junction			Left					Right		
7		С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	0	0	0	0	1	2	0	0	0	1
AM Peak	Max	0	0	0	0	2	4	0	0	0	1
	Average	0	0	0	0	3	3	0	0	0	1
PM Peak	Max	0	0	0	0	5	4	0	0	0	1
				Left					Right		
Junction G	to Junction H	С	LGV	HGV	В	М	С	LGV	HGV	В	М
	Average	9	1	0	0	0	0	0	0	0	1
		-	1	-	-			-	-		
AM Peak	Max	9	1	0	0	1	1	0	0	0	3
AM Peak PM Peak		9	1	0	0	0	1	0	0	0	2

1				Left					Right		
Junction I t	o Junction 29	С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	2	0	0	0	1	0	0	0	0	0
AM Peak	Max	2	0	0	0	1	0	0	0	0	0
D14 D	Average	3	0	0	0	1	0	0	0	0	0
PM Peak	Max	4	1	0	0	2	0	0	0	0	0
Junction 32	to Junction			Left					Right		
31		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	3	0	0	0	1	5	0	0	0	0
AM Peak	Max	6	0	0	0	2	7	0	0	0	1
DM D I	Average	4	0	0	0	0	8	0	0	0	0
PM Peak	Max	5	0	0	0	0	9	0	0	0	0
Junction 33	to Junction			Left		•			Right		
J		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	0	0	0	0	1	2	0	0	0	0
AM Peak	Max	1	0	0	0	2	6	0	0	0	0
D14 D	Average	8	0	0	0	1	5	0	0	0	1
PM Peak	Max	7	0	0	0	2	6	0	0	0	2
Junction H	to Junction			Left					Right		
24		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	4	0	0	0	4	3	0	0	0	0
AM Peak	Max	4	0	0	0	5	4	0	0	0	1
DM D I	Average	5	0	0	0	1	10	0	0	0	1
PM Peak	Max	6	0	0	0	2	11	0	0	0	1
Junction 35	to Junction			Left					Right		
Н		С	LGV	HGV	В	М	С	LGV	HGV	В	М
AAA Daala	Average	0	0	0	0	0	0	0	0	0	1
AM Peak	Max	1	0	0	0	0	0	0	0	0	2
DM Daal	Average	7	0	0	0	0	9	0	0	0	2
PM Peak	Max	5	0	0	0	0	9	0	0	0	4
Junction G	to Junction		•	Left					Right		
74		С	LGV	HGV	В	М	С	LGV	HGV	В	М
ANA Deed	Average	7	0	0	0	2	0	0	0	0	0
AM Peak	Max	7	0	0	0	3	0	0	0	0	0
DM D. I	Average	6	1	0	0	1	0	0	0	0	0
PM Peak	Max	7	1	0	0	2	0	0	0	0	0

4.2.2 OFF-STREET PARKING OCCUPANCY

Off-street parking occupancy surveys were conducted at three large off-site parking facilities within the study area, namely Lebuh Pantai Multi-storey Parking, Tun Syed Off-Street Parking, Union Off-Street Parking locations. The locations were shown in Figure 38.

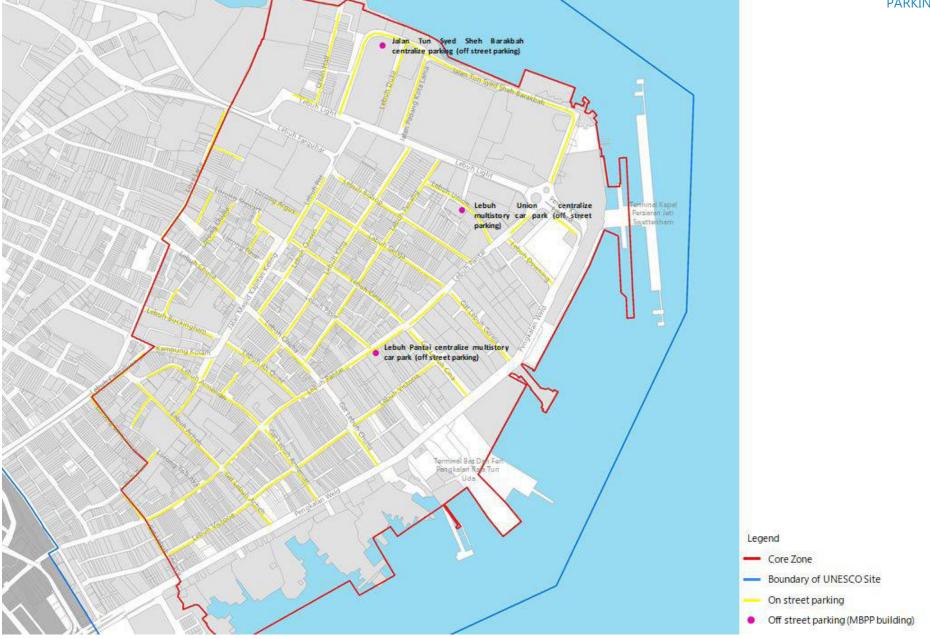


Figure 38: Off-street Parking Survey Locations

Different to on-street parking, off-street parking sites have defined ingress and egress points, which makes it possible to monitor the parking occupancy throughout the survey period with one reference occupancy counts at the beginning of the survey period, and continuous counting of vehicles entering and exiting the parking facility. Thus, the results reported in Tables 7 to 9 below show the number of vehicle entering, leaving, or remaining in the car park.

Table 7: Lebuh Pantai Multi-storey Parking

AM Dools		I	N			0	UT			occui	PANCY	
AM Peak	Car	Van	Lorry	МС	Car	Van	Lorry	MC	Car	Van	Lorry	MC
8:00	8	0	0	2	1	0	0	0	13	0	0	3
9:00	28	1	0	6	2	0	0	1	39	1	0	8
10:00	31	0	0	1	3	0	0	1	67	1	0	8

AM Dools		I	N			0	UT			occui	PANCY	
AM Peak	Car	Van	Lorry	MC	Car	Van	Lorry	MC	Car	Van	Lorry	МС
17:30	14	0	0	1	40	0	0	3	71	1	0	4
18:30	3	0	0	2	33	0	0	4	41	1	0	2
19:30	2	0	0	1	22	0	0	2	21	1	0	1

Table 8: Tun Syed Off Street Parking

AM Dook		II	N			0	UT			occui	PANCY	
AM Peak	Car	Van	Lorry	MC	Car	Van	Lorry	MC	Car	Van	Lorry	MC
8:00	26	0	0	38	0	0	0	1	28	1	2	37
9:00	21	0	0	30	3	0	0	1	46	1	2	66
10:00	3	0	0	0	2	1	0	3	47	0	2	63

AM Peak		I	N			0	UT			occui	PANCY	
Alvi Peak	Car	Van	Lorry	MC	Car	Van	Lorry	MC	Car	Van	Lorry	МС
17:30	1	0	0	1	9	0	0	15	16	0	2	15
18:30	0	0	0	1	11	0	0	11	5	0	2	5
19:30	0	0	0	0	3	0	0	5	2	0	2	0

Table 9: Union Off Street Parking

AM Peak		I	N			0	UT			occui	PANCY	
AIVI Peak	Car	Van	Lorry	МС	Car	Van	Lorry	MC	Car	Van	Lorry	MC
8:00	31	0	0 3		0	0	0	1	35	0	0	3
9:00	90	0	0	0	1	0	0	1	124	0	0	2
10:00	46	0	0	0	1	0	0	1	169	0	0	1

AM Dook		I	N			O	UT			occui	PANCY	
AM Peak	Car	Van	Lorry	MC	Car	Van	Lorry	МС	Car	Van	Lorry	МС
17:30	3	0	0	6	30	0	0	1	119	0	0	1
18:30	0	0	0	1	52	0	0	2	67	0	0	0
19:30	0	0	0	0	50	0	0	0	17	0	0	0

4.2.3 PARKING DWELL TIME SURVEY

On-street parking is prevalent in many parts of Georgetown. The movement of vehicles into and out of on-street parking locations is a major contributor to regular congestion. Therefore, it is important for the micro-simulation model to survey the average time of dwell for vehicles utilising the on-street parking. This provides an insight into the behaviour of vehicle parking and the turnaround rate for the parking facility.

Road sections marked in purple in Figure 39 were pre-identified as popular sections for the parking dwell time survey to take place. The road sections were labelled from DP1 to DP21.

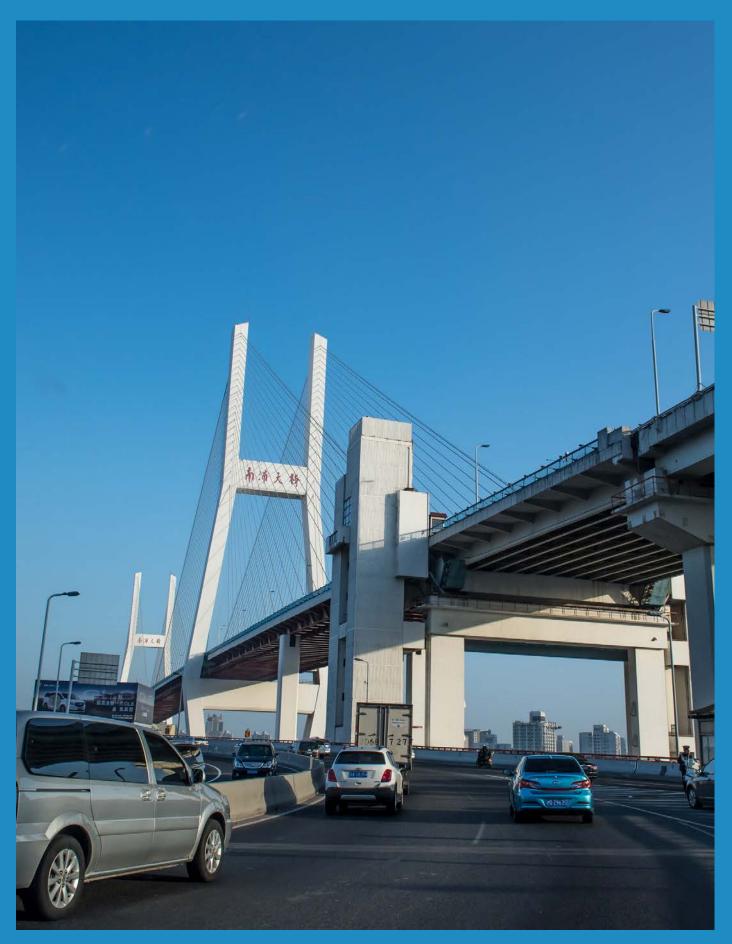


Figure 39: Parking Dwell Time Survey Locations

Due to site constraints dwell time parking surveys for location-6 required two camera locations to view accurate dwell time results. Therefore two cameras were setup. One was located at J21 towards J15 and other was located at J3 towards J15 Results for parking dwell time survey are shown in the table below.

Table 10: Parking Dwell Time Survey Results

Location	Parking Dwell Time (Average) hh:mm:ss	Standard Deviation
DP1	00:00:57	0.001423627
DP2	00:02:48	0.002659139
DP3	00:02:24	0.002374612
DP4	00:01:06	0.001725286
DP5	00:00:48	0.011774317
DP6 (start cam)	00:00:44	0.004050309
DP6 (end cam)	00:02:10	0.020866345
DP7	00:04:01	0.00308549
DP8	00:01:33	0.001934816
DP9	00:01:54	0.001611781
DP10	00:00:30	0.001211326
DP11	00:04:50	0.036085224
DP12	00:01:03	0.001702346
DP13	00:00:31	0.001151509
DP14	00:02:58	0.002703913
DP15	00:00:05	0.001152124
DP16	00:00:40	0.005782647
DP17	00:03:43	0.003185964
DP18	00:16:49	0.195185765
DP19	00:02:30	0.002608574
DP20	00:00:54	0.001700805
DP21	00:01:15	0.001764388



ORIGIN-DESTINATION DATA SURVEY



Origin-Destination Travel Pattern Data can be used to approximate the travel patterns within the model area for more accurate representation of the on-site conditions. All major entry and exit points to the study area were used for collection of origin-destination travel pattern data for this survey. Details of the origindestination survey are shown in the sections below.

5.1 SURVEY ZONING SYSTEM

The zoning system that is adopted for the study area is as shown in Figure 40. The zones are in reference to collection of travel pattern data type as referenced in Section 2.4.1 of the report. Zones were selected to represent the entry and exit points of traffic models, where traffic arrives and departs the network. Traffic will flow from one to another, on occasions spending time in parking spaces.

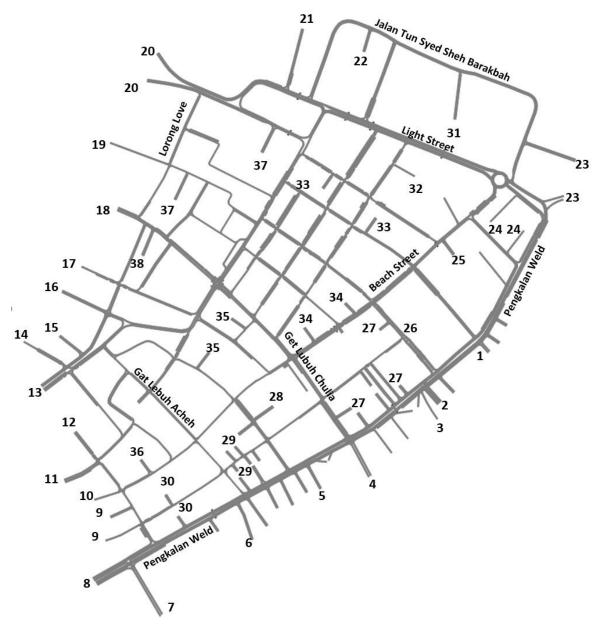


Figure 40: Survey Zoning System

Table 11: Zone Numbering and Description

Zone Number	Zone Description
1	Jalan Gereja
2	Exit from Ferry Terminal
3	Entry to Ferry Terminal
4	Local Road
5	Local Road
6	Road to Tan Jetty Thai Food
7	Raya Merdeka Highway
8	Pengkalan Road
9	Lebuh Victoria Road
10	Lebuh Pantai
11	Lorong Ikan
12	Lebuh Melayu
13	Lebuh Carnarvon
14	Lebuh Kimberley
15	Lorong Ngah Aboo
16	Pesara Claimant
17	Lebuh Campbell
18	Chulia Street
19	Muntri Street
20	Lebuh Farquhar
21	Jalan Green Hall
22	Lebuh Duke
23	Access to Cruise Terminal
24	Private Car Park Access
25	Access to Local Roads
26	Access to Pusaka Warison
27	Access to Local Roads
28	Access to Local Roads
29	Access to Local Roads
30	Access to Local Roads
31	Access to Local Roads
32	Lebuh Union
33	Access to Local Roads
34	Access to Local Roads
35	Access to Local Roads
36	Access to Local Roads
37	Access to Local Roads
38	Lorong Cheapside

5.2 ORIGIN-DESTINATION SURVEY UNIT MATRIX

The representative unit matrix from the TomTom data that was utilized in the matrix estimation and correction procedure is as given below in Table 12 with each row title representing an origin and each column title representing a destination.

Unit matrix is required to represent the connectivity between zones. A value of 0 means there would not be any traffic flow between the pair of origin and destination, either due to inaccessibility or no demand. A value of one means the origin and destination pair is feasible.

These correspond with the zone locations shown in Figure 40 and Table 11.

Table 12: Unit Matrix (with row title representing an origin and column title representing a destination)

Zone	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38
1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	1	0	1	0	1	1	1	0	0	0	0	1	0	0	0	0	1	0
2	0	0	1	1	0	0	0	1	0	0	0	0	1	1	1	0	0	1	0	1	1	1	1	1	1	1	1	0	0	0	0	1	0	1	0	1	1	1
3	1	1	0	0	0	0	1	1	0	0	0	0	1	1	0	0	0	1	0	1	1	0	0	1	0	0	1	0	0	0	0	1	0	1	0	0	1	0
4	0	0	1	0	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1	1	0	1	0	1	1	0
5	0	0	0	0	0	0	1	1	0	1	0	0	0	0	0	1	0	1	0	1	0	0	0	1	1	1	0	0	1	0	0	1	0	0	1	0	0	0
6	0	0	0	0	0	0	1	1	0	1	1	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0
7	1	1	1	1	1	1	0	1	1	1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	1	1
8	1	1	1	1	1	1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
9	0	0	0	0	1	1	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1	1	0	1	0	0	1	0	0	0
10	1	1	1	0	1	0	1	1	0	0	1	1	1	1	1	0	0	1	1	1	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
11	0	1	0	0	0	0	0	1	0	0	0	1	1	1	1	0	0	1	0	1	1	1	0	0	1	1	1	0	0	1	0	1	0	0	1	1	1	1
12	0	1	0	0	0	0	0	1	0	0	0	0	1	1	0	0	0	0	1	1	1	0	0	0	0	1	1	1	0	0	1	1	1	1	0	1	1	1
13	0	1	1	1	0	0	1	1	1	1	0	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
14	0	1	0	0	0	0	1	1	1	0	0	0	1	0	0	1	1	1	0	1	0	1	0	0	1	1	0	1	1	0	0	1	0	1	0	1	1	1
15	0	0	1	0	0	1	1	1	0	0	0	0	1	1	0	1	1	1	0	1	0	0	0	0	0	0	0	0	1	1	0	1	0	1	0	1	1	1
16	0	0	0	0	1	0	1	1	0	1	0	0	1	1	1	0	1	1	0	1	0	0	0	0	0	0	0	0	1	1	0	1	0	1	1	1	1	1
17	0	1	0	1	0	0	1	1	0	0	1	1	1	1	1	1	0	1	1	1	1	1	0	1	1	1	1	1	1	0	0	1	1	1	1	1	1	1
18	1	1	1	0	1	0	1	1	1	1	0	1	1	1	1	1	1	0	1	1	1	0	0	1	1	1	1	1	1	0	1	1	1	1	1	1	1	1
19	0	1	1	1	0	1	1	1	0	0	0	1	1	1	0	1	1	1	0	1	0	0	1	1	1	1	1	0	0	0	1	1	1	1	1	1	1	0
20	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
21	0	1	1	0	1	1	1	1	0	1	1	0	1	0	1	0	0	1	1	1	0	1	1	1	1	1	0	0	1	0	1	1	1	0	0	0	1	0
22	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	1	1	0	1	1	0	1	1	0	1	0	0	0	0	1	1	0	0	0	0	0	0
23	0	0	1	0	0	0	1	1	0	0	0	0	0	0	0	0	0	1	1	1	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	1	1
24	0	1	1	1	1	0	1	1	0	0	0	1	1	1	0	0	0	1	0	1	1	1	1	0	1	0	1	1	0	1	1	1	0	1	1	1	1	1
25	0	1	1	1	0	0	1	1	0	0	1	0	1	1	1	0	0	1	1	1	1	0	0	1	0	1	1	0	0	0	1	1	0	0	1	1	1	0
26	0	0	1	0	0	0	1	1	0	0	0	0	1	1	0	0	0	1	0	1	1	0	1	0	1	0	1	0	1	1	1	1	1	1	1	0	1	0
27	0	1	1	1	0	0	1	1	0	0	0	1	1	1	1	0	1	1	1	1	0	0	1	1	1	0	0	0	0	1	1	1	1	1	1	0	1	0
28	0	1	0	0	1	0	1	1	1	0	0	1	1	1	1	1	0	1	1	1	1	0	0	0	1	1	1	0	1	1	0	1	1	0	1	1	1	1
29	1	1	0	0	0	0	1	1	0	0	0	0	1	1	0	0	1	1	0	1	1	0	1	1	0	0	1	1	0	0	1	1	0	1	1	1	1	0
30	0	1	0	0	0	0	1	1	1	1	0	0	1	1	1	0	1	1	0	1	0	0	1	0	1	0	1	0	1	0	1	1	1	1	0	0	1	0
31	0	1	1	0	0	1	1	1	0	0	0	0	1	0	0	0	0	1	0	1	1	1	1	0	1	0	1	0	0	0	0	0	1	1	0	1	0	0
32	1	1	1	1	1	1	1	1	0	0	0	1	1	1	1	0	1	1	1	1	1	1	1	1	1	1	1	0	1	0	0	0	0	1	1	1	1	0
33	0	1	0	0	0	0	1	1	0	0	0	0	1	0	1	0	0	1	0	1	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	1	1
34	0	1	0	0	0	0	0	1	0	0	0	0	1	1	0	0	0	1	1	1	0	0	0	0	1	1	1	0	0	1	0	1	1	0	1	1	1	0
35	0	1	0	0	0	0	1	1	0	0	1	0	1	1	0	0	1	1	0	1	0	0	0	0	1	1	1	0	1	1	0	1	0	1	0	1	1	1
36	0	0	0	1	0	0	1	1	0	0	0	1	1	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	1	1
37	1	1	1	1	1	0	1	1	1	0	1	1	1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	1
38	0	1	1	0	1	0	1	1	0	1	1	1	1	1	1	0	1	1	0	1	1	0	1	0	0	1	1	1	1	0	0	1	1	1	1	0	1	0

5.3 ORIGIN-DESTINATION SURVEY RESULT MATRIX

TomTom data extracted for the AM peak between 7:00am and 10:00am during Tuesday, Wednesday, and Thursday of November 2021 (consistent with the on-site traffic survey time period) is as shown in Table 13.

Table 13: TomTom Matrix

1	Zone	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38
					_		_						_			_	_																	_					0
3		_	-		<u> </u>	-	<u> </u>	-		-		-	_	-	-	H	<u> </u>		-	_	-	_	-	_		_	<u> </u>			-	-	-		<u> </u>	-	H		-	2
4	3	1	3	0	_	0		_	_	0			_		_	0	0	0	_	_			0	_	2		0	1	0	0		0		_		0	0		0
Fig.	4	0	0	1	0	0	0	4	14	0	0	0	0		0	0	0	0	0	0	3	0	0	0		0	0	0	0	0	0	1		0	1	0	1	4	0
The color The	5	0	0	0	0	0	0	13	8	0	1	0	0	0	0	0	1	0	1	0	2	0	0	0	1	3	1	0	0	2	0	0	2	0	0	2	0	0	0
8	6	0	0	0	0	0	0	20	13	0	1	1	0	3	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	2	0	3	0
Part	7	1	2	1	5	6	14	0	293	4	1	2	0	12	9	1	1	3	6	8	50	9	3	1	18	26	9	32	10	15	5	8	37	6	4	3	0	24	2
The column The	8	19	51	16	20	16	15	94		33	28	8	43	255	144	30	18	24	131	28		136	65	40	206	253	121	251	76	288	120	57	536	91	140	79	73	421	92
11	9	0	0	0	0	1	1	5	16	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1	4	0	1	0	0	2	0	0	0
12	10	1	3	1	0	2	0	2	25	0	0	1	4	18	7	1	0	0	7	1	21	0	0	0	7	11	2	10	12	14	15	1	11	7	5	13	19	4	1
13	11	0	1	0	0	0	0	0	3	0	0	0	4	7	1	1	0	0	1	0	3	4	1	0	0	1	2	1	0	0	1	0	2	0	0	1	2	8	2
14	12	0	1	0	0	0	0	0	7	0	0	0	0	14	2	0	0	0	0	1	5	1	0	0	0	0	1	1	1	0	0	1	5	2	2	0	1	2	3
15	13	0	20	6	2	0	0	24	967	2	22	0	13	0	206	19	13	17	96	8	165	19	11	2	10	19	10	21	13	20	12	9	95	20	52	33	63	102	222
16	14	0	4	0	0	0	0	4	20	2	0	0	0	19	0	0	3	1	6	0	6	0	1	0	0	4	4	0	1	1	0	0	5	0	7	0	1	5	20
17	15	0	0	1	0	0	2	3	11	0	0	0	0	3	2	0	1	1	1	0	1	0	0	0	0	0	0	0	0	1	1	0	1	0	1	0	1	1	3
18	16	0	0	0	0	1	0	4	8	0	1	0	0	3	5	8	0	1	2	0	1	0	0	0	0	0	0	0	0	1	3	0	3	0	2	1	1	2	1
19	17	0	6	0	1	0	0	10	81	0	0	1	5	38	28	46	5	0	23	2	23	1	3	0	2	7	1	7	3	2	0	0	6	2	4	6	12	9	35
20	18	1	10	12	0	1	0	11	76	2	1	0	4	20	15	6	2	4	0	3	40	1	0	0	4	5	1	10	7	2	0	2	7	6	9	10	12	26	12
21 0 6 3 0 2 1 35 87 0 3 1 0 1 0 0 1 1 0 0 0 1 1 0 0 0 1 1 0 0 0 1 1 0 0 0 1 1 0 0 0 1 1 0 0 0 1 0	19	0	3	2	1	0	1	2	19	0	0	0	1	7	5	0	2	1	3	0	27	0	0	1	1	2	2	1	0	0	0	1	16	8	4	1		22	0
22 0 0 0 0 0 0 0 1 0 0 0 11 0 0 0 11 0 0 0 0 0 0 0 0 0 0 0 0 0 1 2 0 0 0 0	20	12	236	33	15	10	6	280		1	4	19	17	85	38	13	6	7	94	23	0	111	33	52	64	166	40	87	19	21	9	65	322	80	85	32	42	_	20
23 0 0 0 1 0 0 0 0 6 55 0 0 0 0 0 0 0 0 0 0 0 0 0			_		0	2	_			0	3	1	_			-	_	0	_	1	16	0		1	5		1			-	-	12		3	0	_		-	0
24 0 2 3 3 3 2 0 10 50 0 0 0 1 2 2 0 0 0 0 0 0 1 2 2 0 0 0 0	22	_	0	0	0	1	0	0	-	0	0	0	0	0	0	0	0	1	2	0	4	1	0	1	1	0	1	0	0	0	0	1	2	0	0	0	0	0	0
25			_		_		-	_					0			-	_		_	_						1	_	2			-			-		0	0		1
26 0 0 0 1 0 0 0 0 3 21 0 0 0 0 1 1 0 0 0 0 0 3 21 0 0 0 0 1 1 1 0 0 0 0 1 1 0 0 24 2 0 1 0 0 5 0 1 0 1 0 1 1 1 1 4 4 1 1 3 0 7 0 0 1 27 0 3 1 1 1 0 0 0 1 1 31 0 0 0 0 1 3 4 1 0 1 0 1 3 3 3 27 0 0 0 1 1 1 4 0 0 0 0 0 0 2 1 1 9 5 4 2 0 1 1 0 2 8 0 1 0 1 0 1 2 0 1 0 1 0 1 0 1 0 1 0 1 0			-		_		_	_	-						_	_	_		_			_		_	_		_							_		-			1
27 0 3 1 1 0 0 0 11 31 0 0 0 1 3 1 0 0 0 0	_		_		_		_	_	_	-			_				_		_					_	_	_								_		-			0
28 0 1 0 0 1 0 0 1 0 8 55 1 0 0 1 7 3 1 1 0 4 1 7 2 0 0 0 0 2 2 3 0 4 1 0 5 1 0 3 1 1 1 0 3 1 1 1 0 4 1 7 2 0 0 0 0 2 2 3 0 4 1 0 5 1 0 3 1 1 1 0 1 1 0 1 1 0 1 1 0 1 1		_	-					_	_						_		_		_					_			_							_		_			0
29			_		_		_			_																_	_							_		-			0
30 0 1 0 0 0 0 0 0 0 3 33 1 1 0 0 0 0 4 1 1 0 0 0 4 1 1 0 0 0 1 1 0 0 0 0			-		_		_	_	_	_			_		_	_	_		_			_		_	_	_	_			-				_		-			1
31 0 2 2 0 0 0 1 8 26 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0			_				_			_							_									-								_		_			
32 2 5 2 1 1 1 3 17 82 0 0 0 1 7 6 1 0 2 11 5 212 6 5 2 3 8 1 3 0 1 0 0 0 0 0 0 0 0 2 2 3 22 0 3 8 1 3 0 1 0 0 0 0 0 0 0 0 0 0 0			-		_		_	_	_				_			_	_		_					_						-	-			_		_		-	-
33 0 1 0 0 0 0 0 1 14 0 0 0 0 0 0 1 14 0 0 0 0		_	-				-		-								-		_					_		-								-		-			
34 0 2 0	-		-		_		-			_			_			-	<u> </u>		_					_			_			-	-	-		<u> </u>		_		-	-
35 0 1 0 0 0 0 0 4 19 0 0 1 0 22 14 0 0 0 2 2 2 2 0 9 0 0 0 0 1 1 3 0 1 2 0 2 0 2 0 2 0 2 2 2 2 3 0 5 0 0 0 0 0 1 1 3 0 1 2 0 2 0 2 0 2 0 2 2 2 3 0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			-						-								_																	-					
36 0 0 0 0 1 0 0 1 49 0 0 0 1 28 17 0 1 0 3 0 5 0 <th></th> <th></th> <th>_</th> <th></th> <th>_</th> <th></th> <th>-</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>-</th> <th>_</th> <th></th> <th>_</th> <th></th> <th></th> <th></th> <th></th> <th>_</th> <th></th> <th></th> <th>_</th> <th></th> <th></th> <th></th> <th>-</th> <th></th> <th></th> <th>-</th> <th></th> <th>-</th> <th></th> <th>-</th> <th>2</th>			_		_		-									-	_		_					_			_				-			-		-		-	2
37 4 29 4 1 1 0 30 100 2 0 4 2 10 5 0 1 4 12 6 349 7 4 4 2 10 6 8 3 1 1 9 12 9 3 2 6 0 3			_		_		-		_	-						_	-		_			_		_	_	-	_							-					1
			-						_	_			_			_															-			_				-	3
	38	0	9	2	0	1	0	5	45	0	1	1	2	34	19	1	0	2	8	0	11	4	0	2	0	0	2	4	1	2	0	0	6	3	3	3	0	7	0

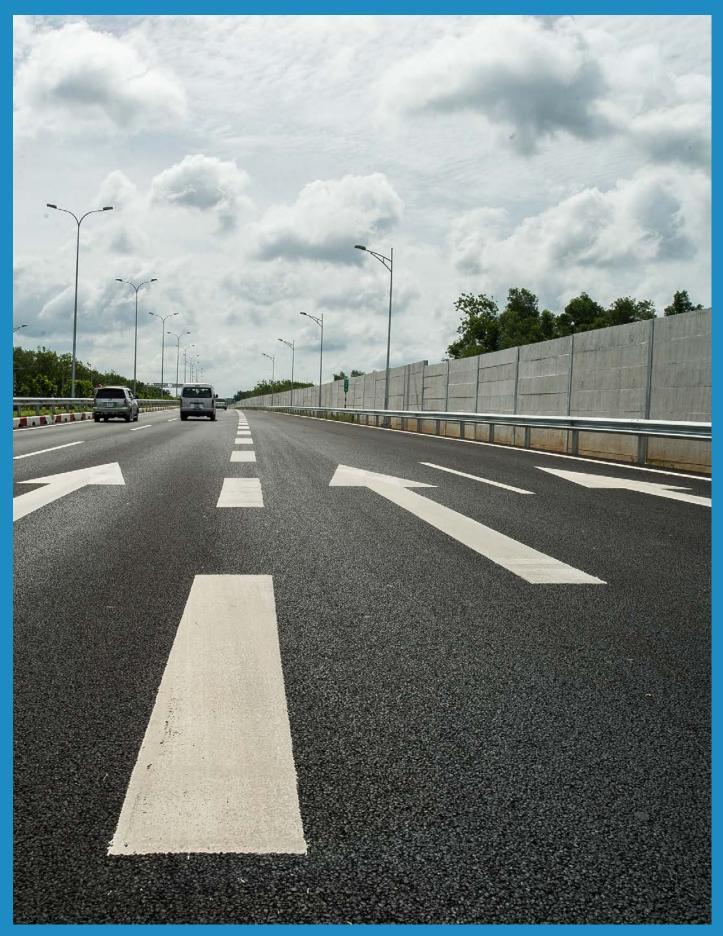
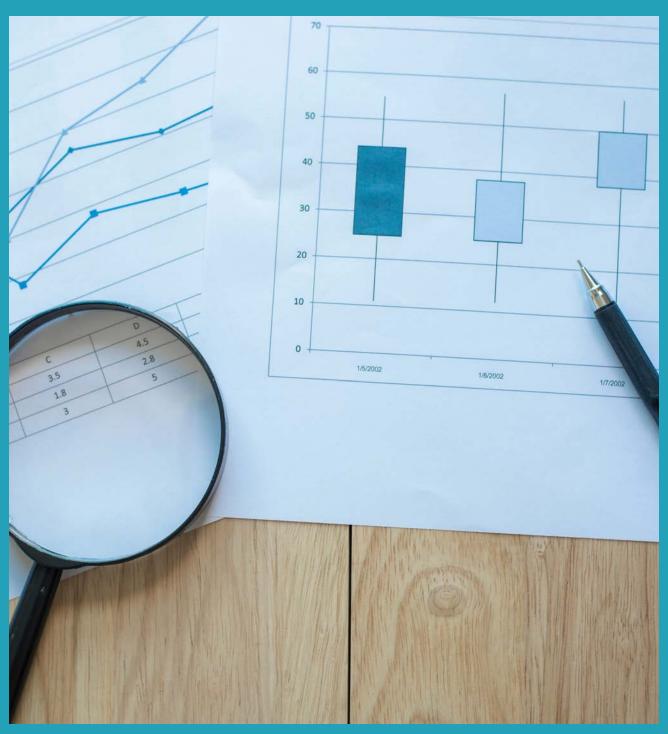


Photo: ADB

SURVEY DATA UTILISATION



6.1 DATA UTILIZATION INTRODUCTION

Site data collected are to be further processed into the format accepted in traffic analysis and modelling. These steps as documented below follow the typical method of how data is utilised for large-scale urban area simulation projects, with customisation made for Penang's on-site conditions.

Data collected are separated into capacity data and survey data. The types of data contained in each category are summarised in Figure 41.

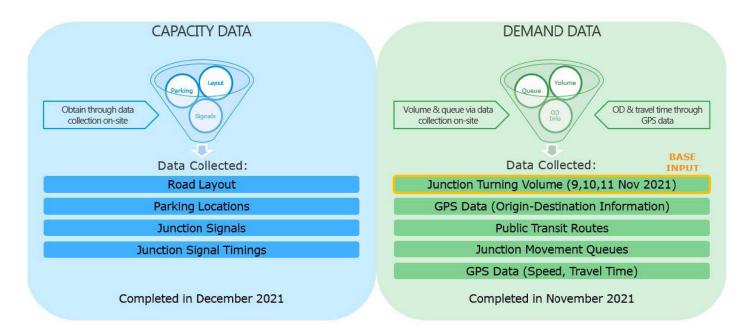


Figure 41: Data Categories

6.2 DATA CONVERSION PROCESS

Raw data collected from site surveys contained three hours of information, and were collected in specific classifications, in individual movements.

In order to complete a complex and dynamic traffic simulation model, the raw data needs to be processed and consolidated into hourly peak data consisting of model classes and with combined movement between zones. The following steps are included to complete the process:

- 1. Conversion of individual vehicle classed to PCU
- 2. Identify peak hours during the survey period
- 3. Produce movement diagram from individual junctions
- 4. Compare movements recorded with allowable ones in road layout
- 5. Check flow balance between upstream and downstream junctions
- 6. Consolidate turn-by-turn movements to OD table

For each of the tasks involved, the following section details the key considerations when completing each step.

6.2.1 CONVERSION TO PCU

Traffic count results were analysed to determine the peak 60-minute periods within the morning and evening peak periods. All traffic flows were converted and expressed in Passenger Car Units (PCUs). PCUs are factors that convert different classification of vehicles to be equivalent to a typical car. The following PCU factors were used (in accordance with Malaysian guidelines) for the junction counts:

• Car: 1.00 • Taxi: 1.00

• Light Goods Vehicles (Lorry Kecil): 2.50 • Heavy Goods Vehicles (Lorry Besar): 3.00

• Bus: 3.00

Motorcycle: 0.75

Traffic flows from different vehicle types need to be converted into Passenger Car Unit (PCU) to be comparable. This step is usually done with Excel spreadsheet calculations with an example shown below.

	Mode						
		Taxi	LGV	HGV	Bus	Motorcycle	Total PCU
Count	626	1	32	0	24	231	
PCU Factor	1	1	2.5	3	3	0.75	952
Total PCUs	626	1	80	0	72	173.25	1

Figure 42: Example of PCU Conversion

6.2.2 IDENTIFY PEAK HOURS

Peak hour is defined as the one-hour period within the survey duration with the highest total amount of traffic among all vehicle movements at all junctions in the network.

Peak hour is identified by adding all vehicle movement counts (per 15-min interval) in PCU values together and picking out the period with highest volume.

As an example, J42 AM hourly traffic is shown below. It can be inferred from the below Figure 43 that peak hour is in between 8 AM to 9 AM.

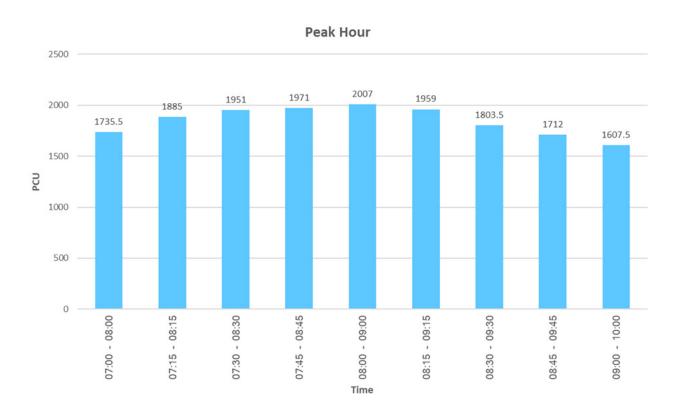


Figure 43: Example of Peak Hour Identification

6.2.3 PRODUCE MOVEMENT DIAGRAM

Once peak hours are identified, the vehicle movements are put in a diagram to show the spatial relations between movements. Movement diagrams produced from this survey have been included in Chapter 3 of this report.

6.2.4 COMPARE WITH ROAD LAYOUT

Data collected on-site contains all vehicle movements happening on-site, regardless of whether the movement is allowed by traffic regulations or not. In order to identify irregular movements, the data collected needs to be compared on individual basis with on-site layout. If any irregularities were discovered, it is to be further studied with either verifying on the counts or modifying the network layout (should the movements occur consistently and in large quantities).

6.2.5 CHECK FLOW BALANCE

Flow between two consecutive junctions is compared to ensure upstream and downstream junctions have consistent flows between themselves.

As an example, J41 and J42 traffic counts are taken and flow balance between the junctions are compared as shown in Figure 44.

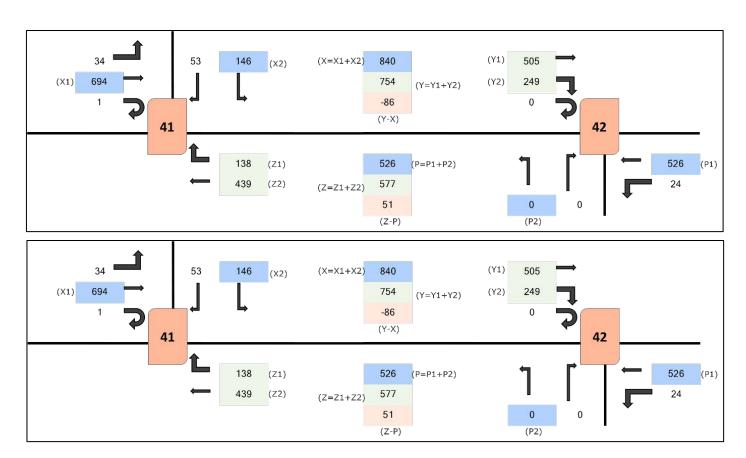


Figure 44: Example of Flow Balance Comparison

Traffic coming out from junction 41 east bound (694+146= 840) is compared with traffic coming to junction 42 east bound (505+ 249= 754). A difference of 86 is observed in east bound direction. Similarly for west bound a difference of 51 is observed.

6.2.6 CONSOLIDATE TO OD TABLE

After flow balance, as a final step OD demand is derived from the turning movement data from the surveys and also from TomTom data using matrix estimation in VISSIM.

Figure 45 shows an example of OD demand. Zone 3 is highlighted in below image and details are explained.

Zone	1	2	3	4	5	Total
1	0	441	219	447	91	1198
2	448	0	408	324	143	1323
3	499	334	0	145	62	1040
4	57	343	141	0	123	664
5	294	291	284	240	0	1109
Total	1298	1409	1052	1156	419	

Figure 45: Example of OD Table Data Source

Sum total of zone 3 highlighted in yellow is originating trips from zone 3 to other zones and cell highlighted in green color is destination trips to zone 3 from other zones.

NEXT STEPS



7.1 NEXT MODEL STAGES

With the survey and data processing being completed following the on-site surveys conducted in November 2021 during the progressive re-opening of Malaysia, the Stage 1 micro-simulation model will be calibrated, and scenario testing will commence.

The Stage 1 micro-simulation report will contain information on the process and use of data in the model calibration and scenario testing.

Upon the acceptance of the Stage 1 micro-simulation model report, Stage 2 will commence which will include simulation of a wider area of Georgetown encompassing the full UNESCO World Heritage area.

On completion of Stage 2, Ramboll will conduct a PTV accredited training courses on the use of VISSIM software for MBPP and Digital Penang in order for the micro-simulation model to be used for ongoing testing of changes to transport within Georgetown beyond the conclusion of this Pilot Project.

7.2 NEXT DELIVERABLE STAGES

With the above model stages, the following future deliverables will be produced and submitted as part of this project.

Table 14: Deliverable Stages

Deliverable	Contents
Stage 1 Base Model Calibration Report (D2A)	Interim Technical Deliverable – This report documents the model development and calibration and is a formal documentation of the models accuracy and reflectiveness of real world conditions
Stage 1 Scenario Testing Report (D2B)	Stage 1 Final Deliverable – This report documents the simulation of the scenario testing and comparison of the base calibrated (real world) model to the future proposed interventions to evaluate their improvement.
Stage 2 Base Model Calibration Report (D3A)	Interim Technical Deliverable (Stage 2) – This report documents the model development and calibration and is a formal documentation of the models accuracy and reflectiveness of real world conditions for the larger Stage 2 area
Stage 2 Scenario Testing Report (D3B)	Stage 2 Final Deliverable – This report documents the simulation of the scenario testing for Stage 2 and comparison of the base calibrated (real world) model to the future proposed interventions to evaluate their improvement.
Final Report (D4) and Project Evaluation (D5)	Compilation of Stage 1 and Stage 2 work above

APPENDICES



APPENDIX 1: SURVEY OBSERVATIONS

A.1.0 EXPLANATION OF SYMBOLOGY

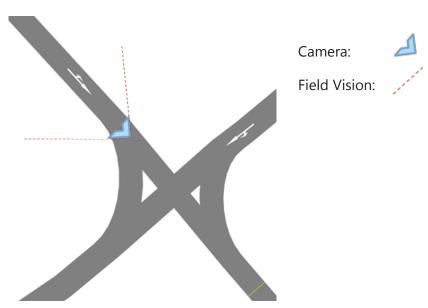


Figure 46: Explanation of symbology

A.1.1 JUNCTION 1: PENGKALAN WELD / LEBUH DOWNING

Junction 1 is a unsignalized T-junction, and the layout is shown in the figure below, followed by the photos taken during site survey. Short queues were observed on approaches of the junction.

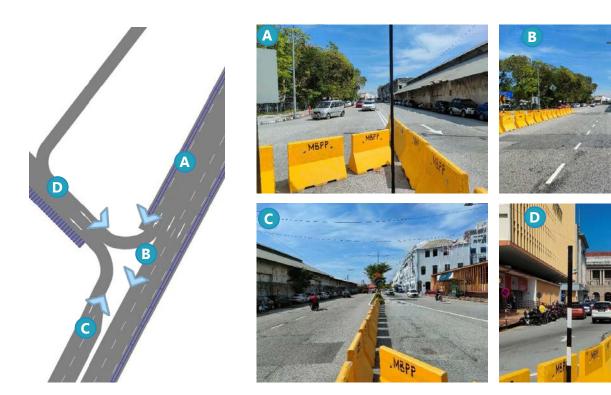


Figure 47: Junction 1 Layout

Figure 48: Junction 1 Trafic Condition

A.1.2 JUNCTION 2: PENGKALAN WELD / GAT LEBUH GEREJA

Junction 2 is a unsignalized T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

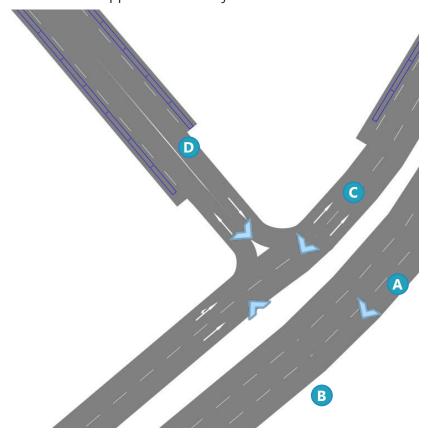


Figure 49: Junction 2 Layout







Figure 50: Junction 2 Traffic Condition

A.1.3 JUNCTION 3: PENGKALAN WELD / GAT LEBUH CHINA

Junction 3 is a unsignalized Double T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

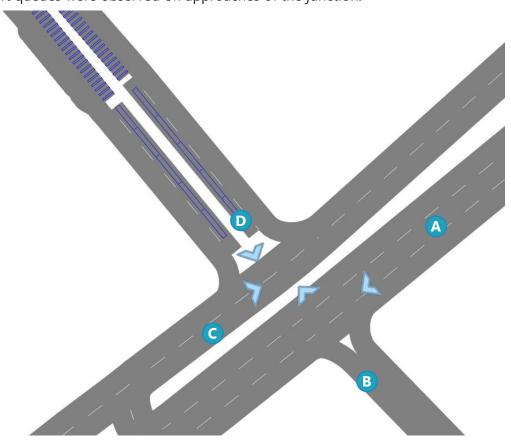


Figure 51: Junction 3 Layout



Figure 52: Junction 3
Traffic Condition

A.1.4 JUNCTION 4: PENGKALAN WELD / GAT LEBUH PASAR

Junction 4 is a signalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium queues were observed at east approach of the junction and short queues were observed at the other approaches of the junction.

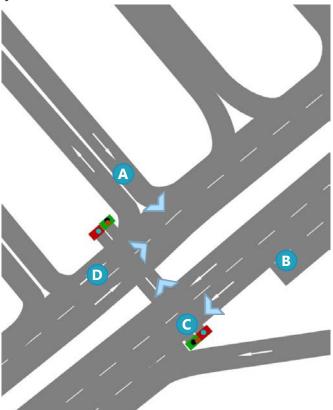


Figure 53: Junction 4 Layout



Figure 54: Junction 4
Traffic Condition

A.1.5 JUNCTION 5: PENGKALAN WELD / GAT LEBUH CHULIA

Junction 5 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on the approaches of the junction.

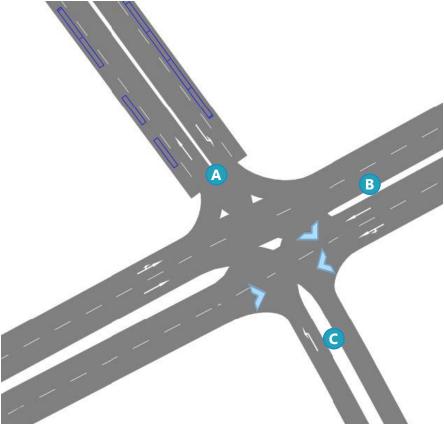


Figure 55: Junction 5 Layout







Figure 56: Junction 5 Traffic Condition

A.1.6 JUNCTION 6: PENGKALAN WELD / GAT LEBUH ARMENIAN

Junction 6 is a unsignalized Double-T junction and the layout is shown in the figure below, followed by the site photos. Long queues were observed at east approach of the junction during the morning and evening peak.

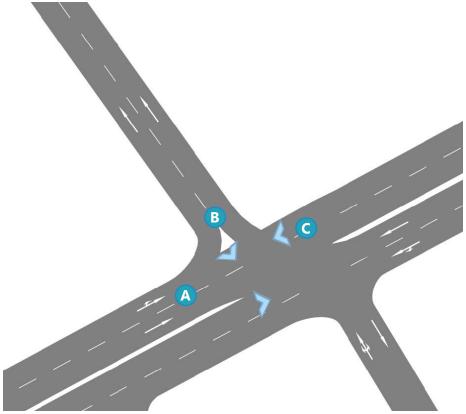


Figure 57: Junction 6 Layout







Figure 58: Junction 6
Traffic Condition

A.1.7 JUNCTION 7: PENGKALAN WELD / GAT LEBUH ACHEH

Junction 7 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Medium queues were observed on all approaches of the junction.

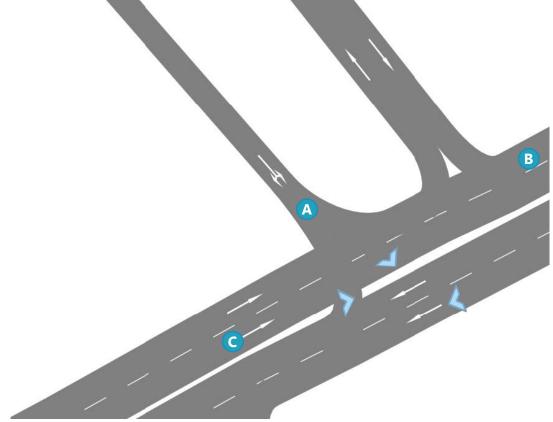


Figure 59: Junction 7 Layout







Figure 60: Junction 7
Traffic Condition

A.1.8 JUNCTION 8: PENGKALAN WELD / RAYA MERDEKA HIGHWAY

Junction 8 is a signalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

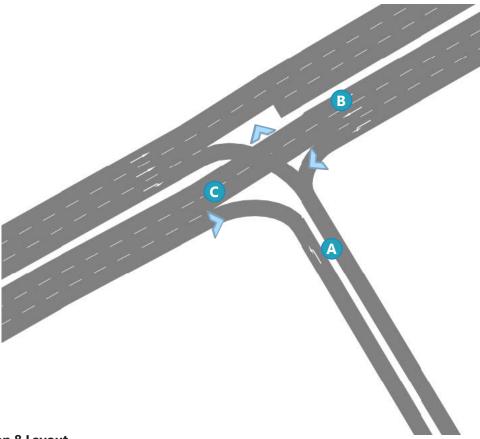


Figure 61: Junction 8 Layout







Figure 62: Junction 8
Traffic Condition

A.1.9 JUNCTION 9: PENGKALAN WELD / GAT LEBUH MELAYU

Junction 9 is a signalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction

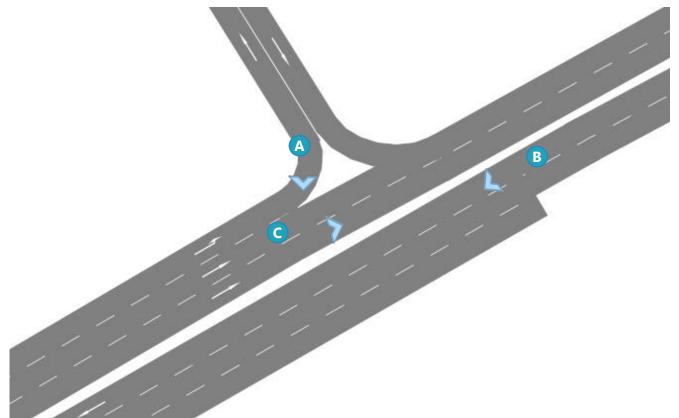


Figure 63: Junction 9 Layout







Figure 64: Junction 9
Traffic Condition

A.1.10 JUNCTION 10: LEBUH VICTORIA / GAT LEBUH MELAYU

Junction 10 is a unsignalized cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction during the morning and evening peak.

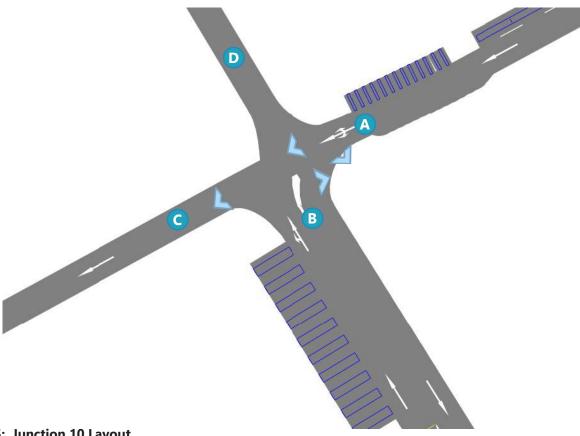


Figure 65: Junction 10 Layout









Figure 66: Junction 10 Traffic Condition

A.1.11 JUNCTION 11: LEBUH VICTORIA / GAT LEBUH ACHEH

Junction 11 is a unsignalized cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

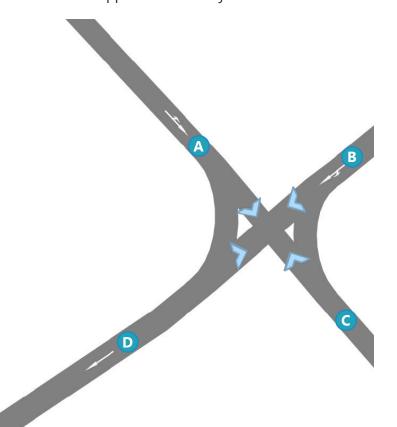


Figure 67: Junction 11 Layout



Figure 68: Junction 11 Traffic Condition

A.1.12 JUNCTION 12: LEBUH VICTORIA / GAT LEBUH ARMENIAN

Junction 12 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

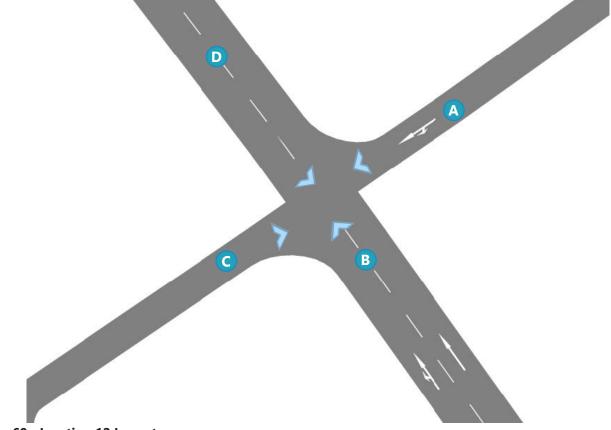


Figure 69: Junction 12 Layout



Figure 70: Junction 12 Traffic Condition

A.1.13 JUNCTION 13: LEBUH VICTORIA / GAT LEBUH CHULIA

Junction 13 is a unsignalized cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

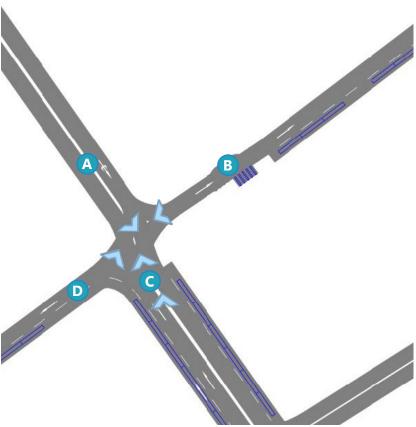


Figure 71: Junction 13 Layout



Figure 72: Junction 13 Traffic Condition

A.1.14 JUNCTION 14: LEBUH VICTORIA / GAT LEBUH PASAR

Junction 14 is a unsignalized T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

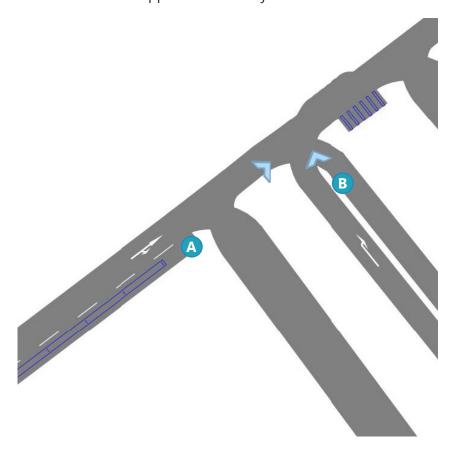


Figure 73: Junction 14 Layout





Figure 74: Junction 14 Traffic Condition

A.1.15 JUNCTION 15: LEBUH VICTORIA / GAT LEBUH CHINA

Junction 15 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

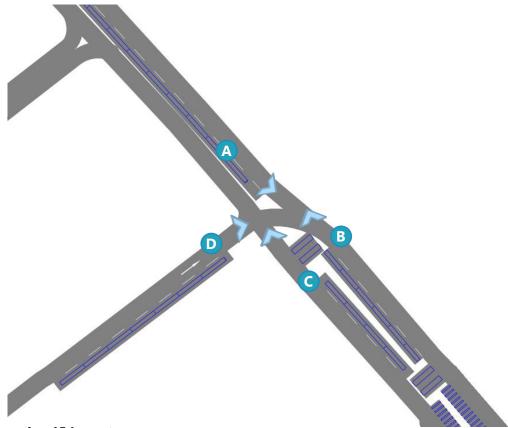


Figure 75: Junction 15 Layout







Figure 76: Junction 15 Traffic Condition

A.1.16 JUNCTION 16: BEACH STREET / PESARA KING EDWARD

Junction 16 is a unsignalized roundabout junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

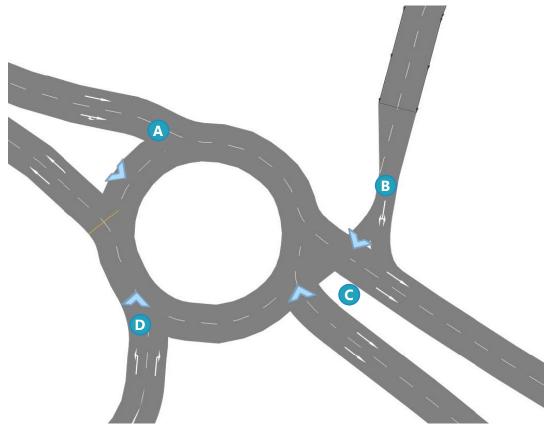


Figure 77: Junction 16 Layout



Figure 78: Junction 16 Traffic Condition

A.1.17 JUNCTION 17: BEACH STREET / LEBUH DOWNING

Junction 17 is a unsignalized T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

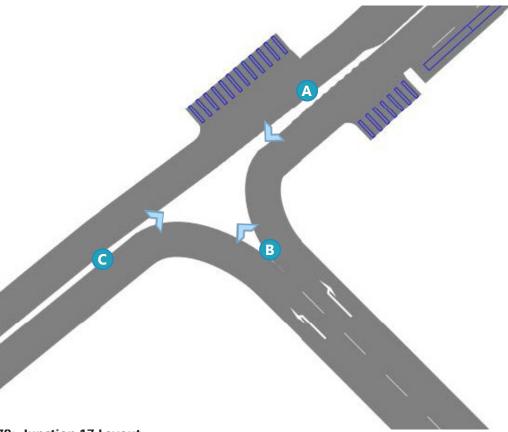


Figure 79: Junction 17 Layout







Figure 80: Junction 17 Traffic Condition

A.1.18 JUNCTION 18: BEACH STREET / LEBUH UNION

Junction 18 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

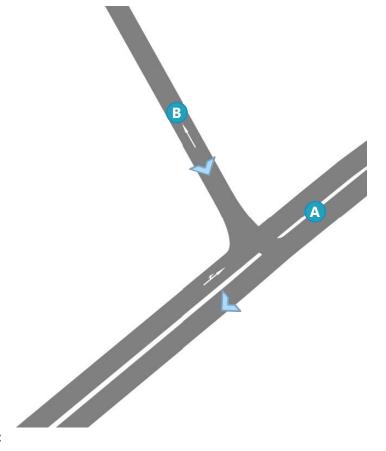


Figure 81: Junction 18 Layout





Figure 82: Junction 18 Traffic Condition

A.1.19 JUNCTION 19: BEACH STREET / BISHOP STREET

Junction 19 is a signalised double T-signalized T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

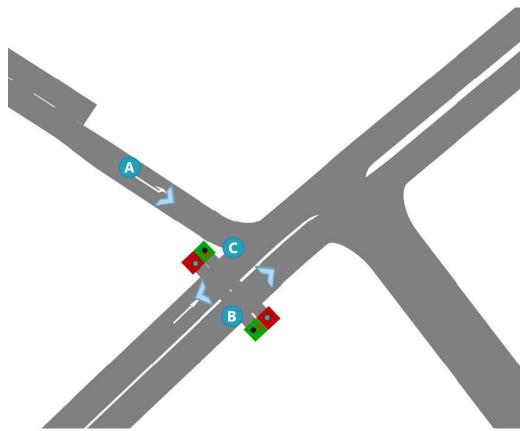


Figure 83: Junction 19 Layout







Figure 84: Junction 19 Traffic Condition

A.1. 20 JUNCTION 20: BEACH STREET / GAT LEBUH GEREJA

Junction 20 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Junction 20 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

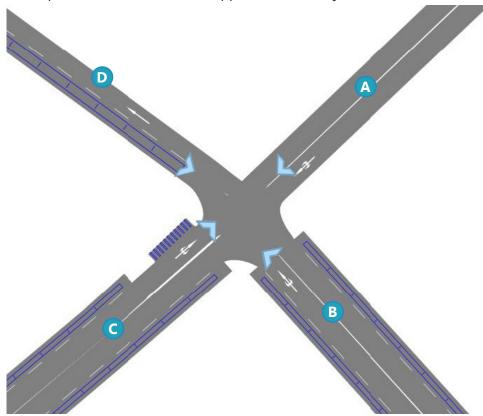


Figure 85: Junction 20 Layout









Figure 86: Junction 20 Traffic Condition

A.1.21 JUNCTION 21: BEACH STREET / GAT LEBUH CHINA

Junction 21 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

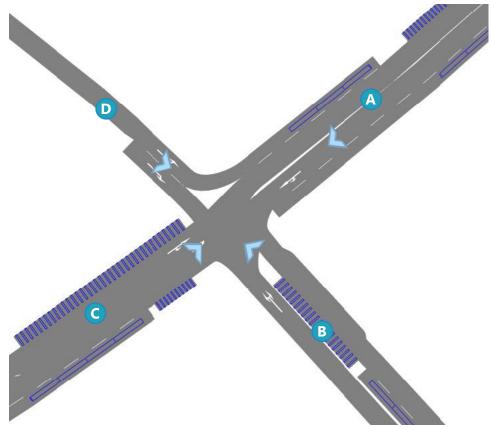


Figure 87: Junction 21 Layout









Figure 88: Junction 21 Traffic Condition

A.1.22 JUNCTION 22: BEACH STREET / LEBUH PASAR

Junction 22 is a signalised T-junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

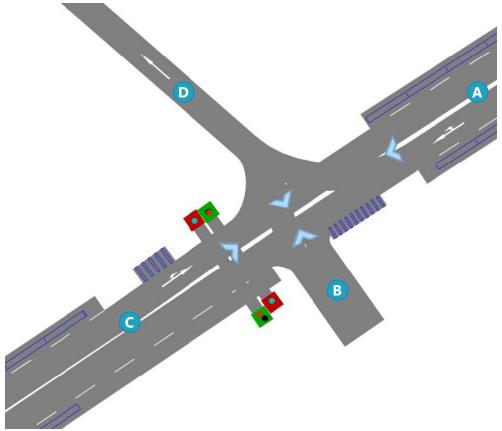


Figure 89: Junction 22 Layout

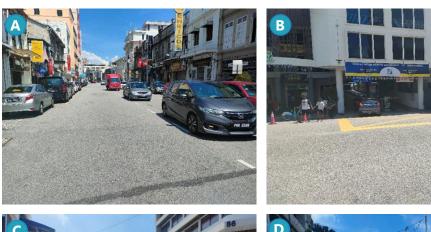






Figure 90: Junction 22 Traffic Condition

A.1.23 JUNCTION 23: BEACH STREET / GAT LEBUH CHULIA

Junction 23 is a signalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

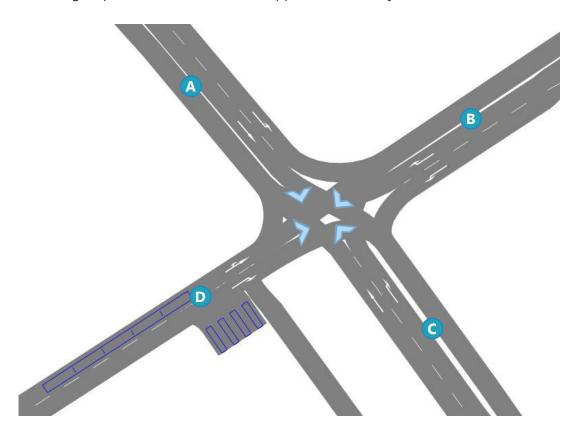


Figure 91: Junction 23 Layout



Figure 92: Junction 23 Traffic Condition

A.1.24 JUNCTION 24: BEACH STREET / LEBUH AH QUEE

Junction 24 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

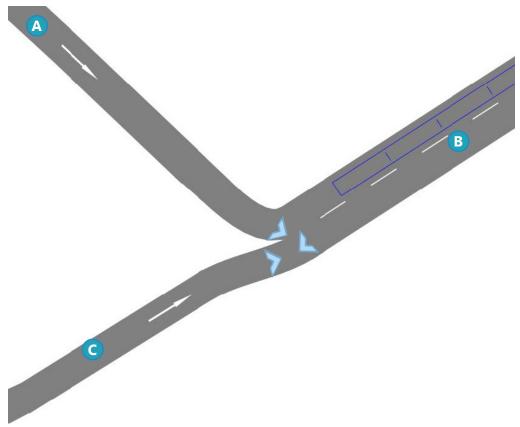


Figure 93: Junction 24 Layout







Figure 94: Junction 24 Traffic Condition

A.1.25 JUNCTION 25: BEACH STREET / GAT LEBUH ARMENIAN

Junction 25 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

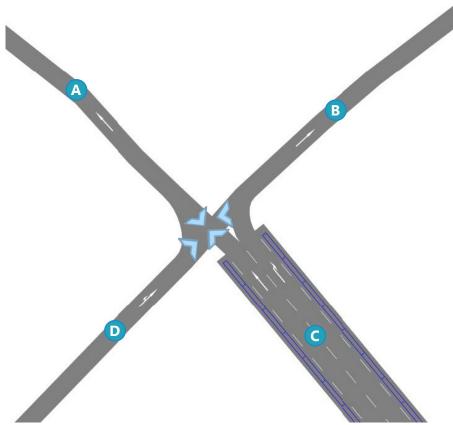


Figure 95: Junction 25 Layout



Figure 96: Junction 25 Traffic Condition

A.1.26 JUNCTION 26: BEACH STREET / GAT LEBUH ACHEH

Junction 26 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

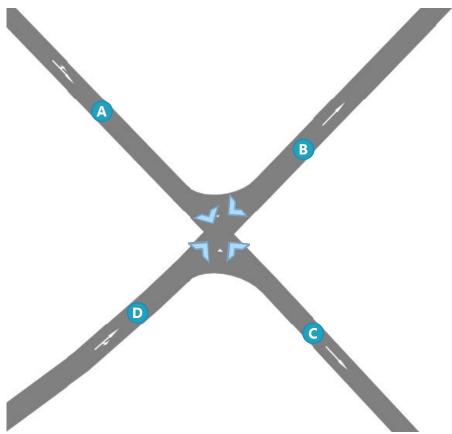


Figure 97: Junction 26 Layout



Figure 98: Junction 26 Traffic Condition

A.1.27 JUNCTION 27: BEACH STREET / GET LEBUH MELAYU

Junction 27 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

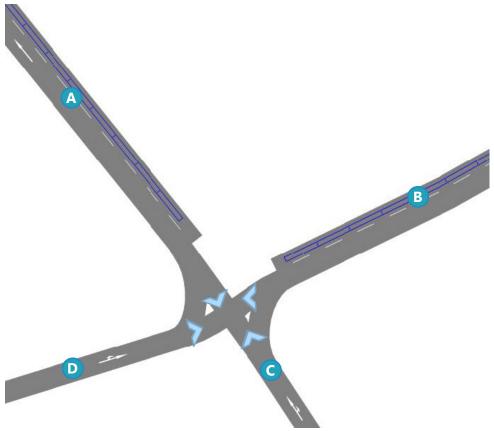


Figure 99: Junction 27 Layout



Figure 100: Junction 27 Traffic Condition

A.1.28 JUNCTION 28: GET LEBUH MELAYU / LORONG TOH AKA

Junction 28 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction..

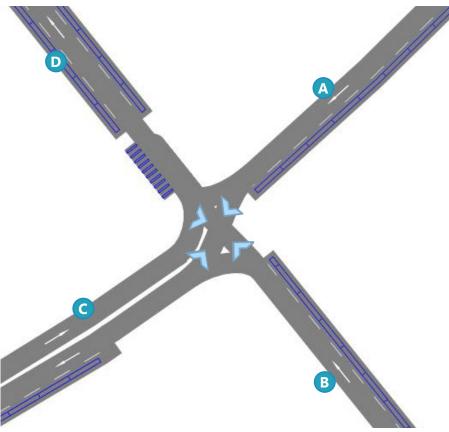


Figure 101: Junction 28 Layout









Figure 102: Junction 28 Traffic Condition

A.1.29 JUNCTION 29: LORONG TOH AKA / LORONG CARNAVON

Junction 29 is a signalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction during the morning and evening peak.

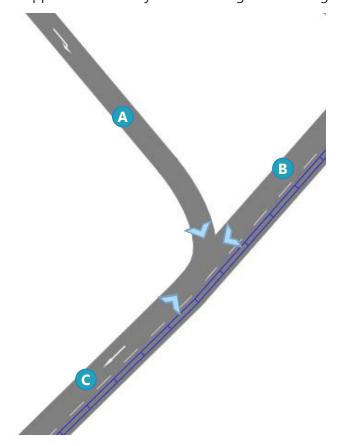


Figure 103: Junction 29 Layout







Figure 104: Junction 29 Traffic Condition

A.1.30 JUNCTION 30: GAT LEBUH ACHEH / LEBUH CARNAVON

Junction 30 is a unsingalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

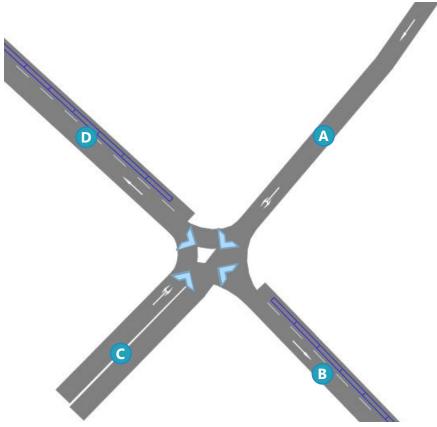


Figure 105: Junction 30 Layout



Figure 106: Junction 30 Traffic Condition

A.1.31 JUNCTION 31: GAT LEBUH ARMENIAN / LORONG CARNAVON

Junction 31 is a unsignalised double T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

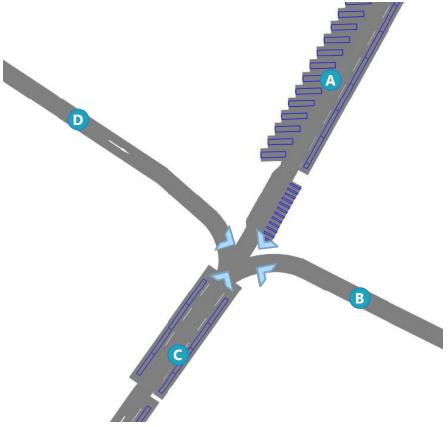


Figure 107: Junction 31 Layout



Figure 108: Junction 31 Traffic Condition

A.1.32 JUNCTION 32: JALAN MASJID KAPITAN KELING / JALAN MASJID KAPITAN KELING

Junction 32 is a unsignalised Y-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

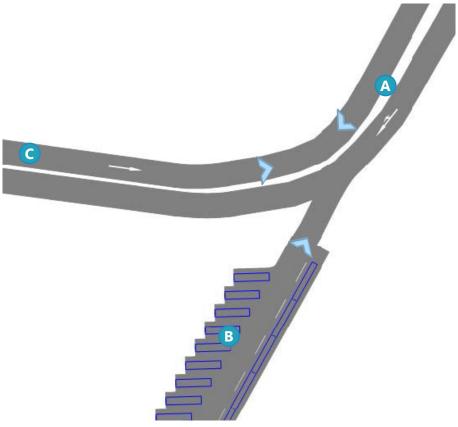


Figure 109: Junction 32 Layout







Figure 110: Junction 32 Traffic Condition

A.1.33 JUNCTION 33: JALAN MASJID KAPITAN KEELING/ LEBUH AH QUEE

Junction 33 is a unsignalized double T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

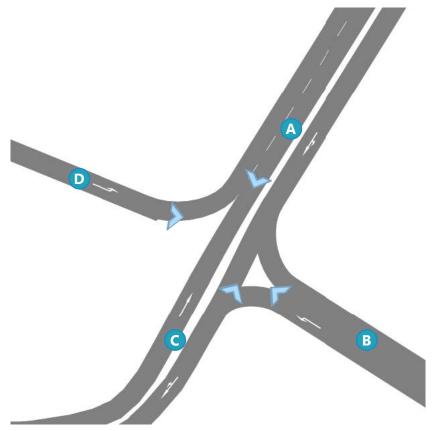


Figure 111: Junction 33 Layout

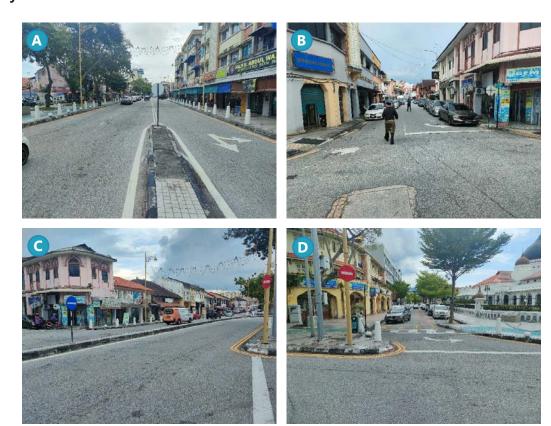


Figure 112: Junction 33 Traffic Condition

A.1.34 JUNCTION 34: JALAN MASJID KAPITAN KELING / CHULIA STREET

Junction 34 is a signalised cross junction, and the layout is shown in the figure below, followed by the site photos. Long queues were observed on approaches of the junction.

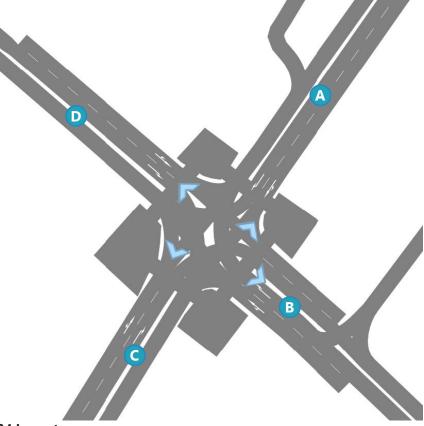


Figure 113: Junction 34 Layout



Figure 114: Junction 34
Traffic Condition

A.1.35 JUNCTION 35: LEBUH KING / CHULIA STREET

Junction 35 is a unsignalised Cross-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

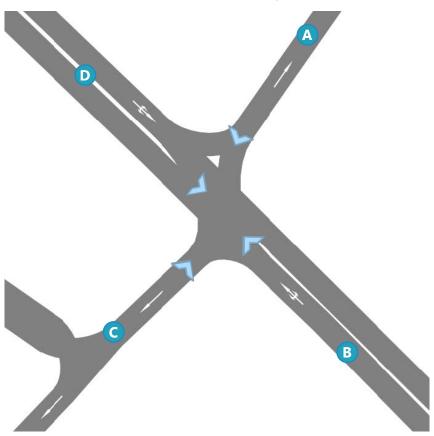


Figure 115: Junction 35 Layout



Figure 116: Junction 35 Traffic Condition

A.1.36 JUNCTION 36: PENANG STREET / CHULIA STREET

Junction 36 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short-length queues were observed on approaches of the junction.

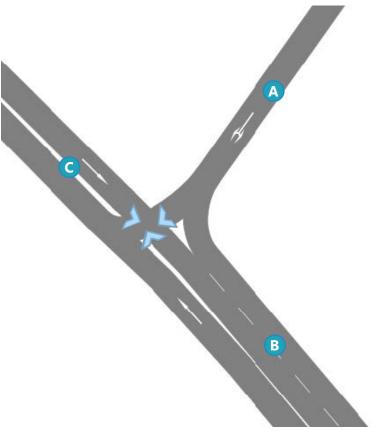


Figure 117: Junction 36 Layout







Figure 118: Junction 36 Traffic Condition

A.1.37 JUNCTION 37: PENANG STREET / LEBUH PASAR

Junction 37 is a unsignalized Cross-junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

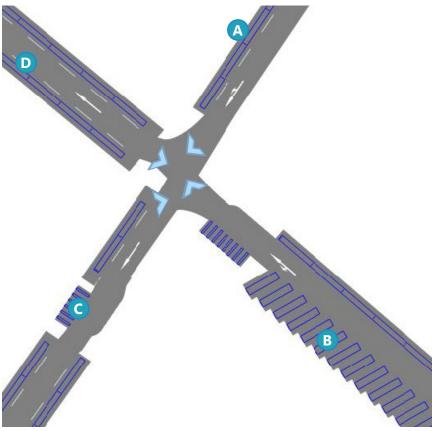


Figure 119: Junction 37 Layout



Figure 120: Junction 37 Traffic Condition

A.1.38 JUNCTION 38: LEBUH KING / LEBUH PASAR

Junction 38 is a unsignalised Cross junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

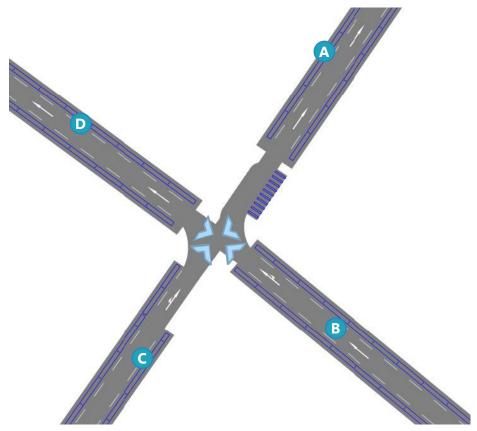


Figure 121: Junction 38 Layout

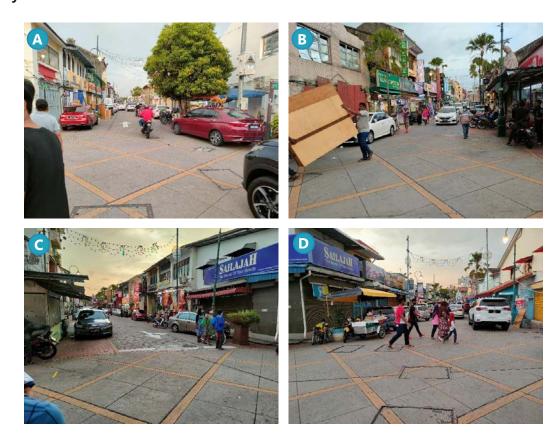


Figure 122: Junction 38 Traffic Condition

A.1.39 JUNCTION 39: LEBUH PASIR / QUEEN STREET

Junction 39 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

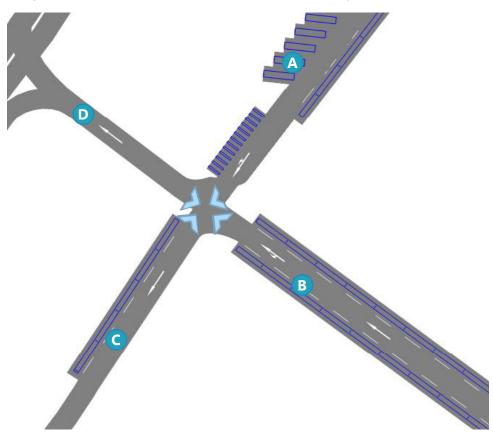


Figure 123: Junction 39 Layout



Figure 124: Junction 39 Traffic Condition

A.1.40 JUNCTION 40: LEBUH PASIR / JALAN MASJID KAPITAN KELING

Junction 40 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

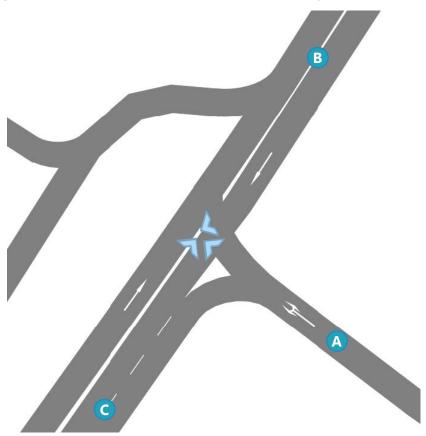


Figure 125: Junction 40 Layout







Figure 126: Junction 40 Traffic Condition

A.1.41 JUNCTION 41: LORONG STEWART / JALAN MASJID KAPITAN KELING

Junction 41 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium length queues were observed on approaches of the junction.

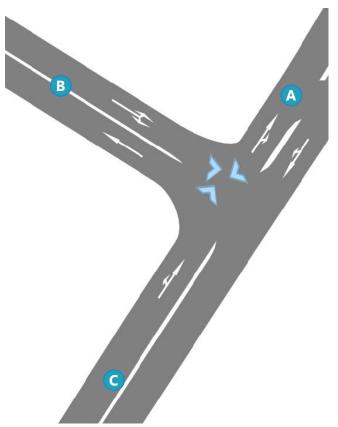


Figure 127: Junction 41 Layout







Figure 128: Junction 41 Traffic Condition

A.1.42 JUNCTION 42: LEBUH CHINA / JALAN MASJID KAPITAN KELING

Junction 42 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

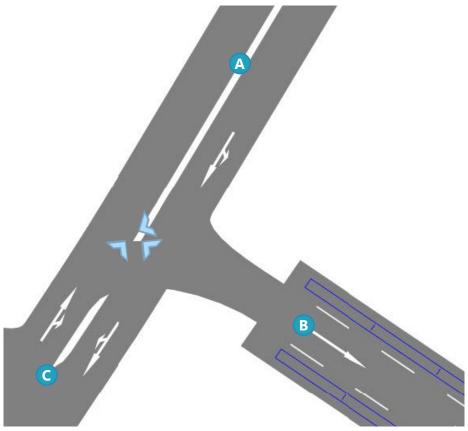


Figure 129: Junction 42 Layout







Figure 130: Junction 42 Traffic Condition

A.1.43 JUNCTION 43: LEBUH CHINA / QUEEN STREET

Junction 43 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

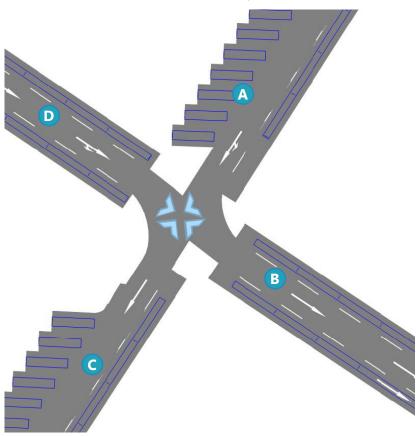


Figure 131: Junction 43 Layout



Figure 132: Junction 43
Traffic Condition

A.1.44 JUNCTION 44: LEBUH CHINA / LEBUH KING

Junction 44 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

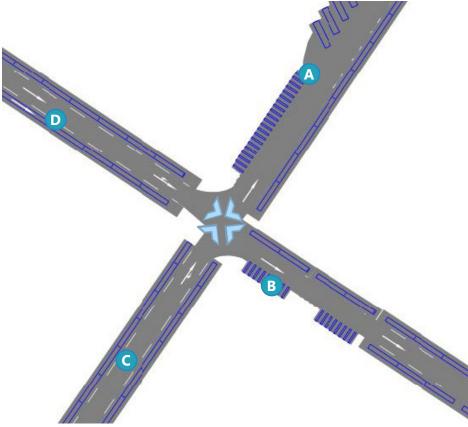


Figure 133: Junction 44 Layout

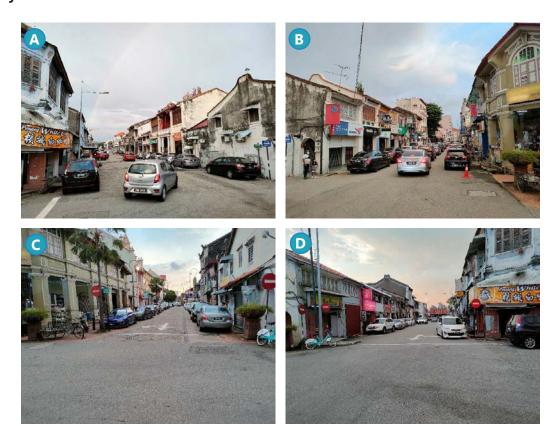


Figure 134: Junction 44 Traffic Condition

A.1.45 JUNCTION 45: LEBUH CHINA / PENANG STREET

Junction 45 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

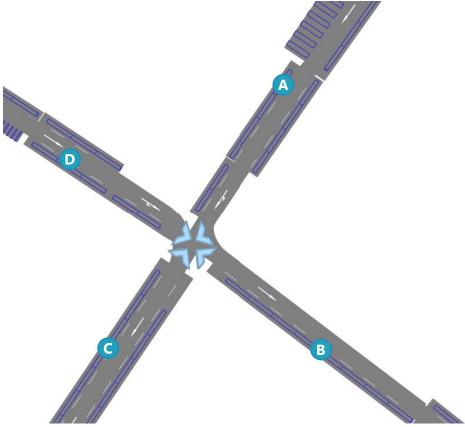


Figure 135: Junction 45 Layout









Figure 136: Junction 45 Traffic Condition

A.1.46 JUNCTION 46: CHURCH STREET / PENANG STREET

Junction 46 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

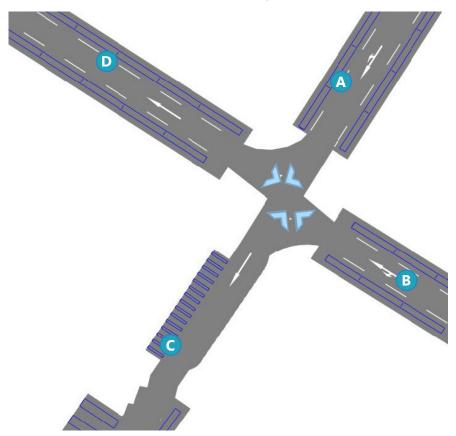


Figure 137: Junction 46 Layout







Figure 138: Junction 46 Traffic Condition

A.1.47 JUNCTION 47: CHURCH STREET / LEBUH KING

Junction 47 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

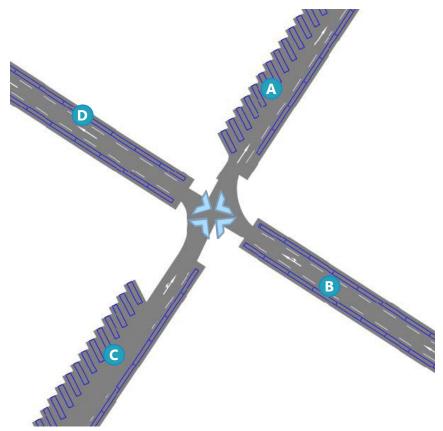


Figure 139: Junction 47 Layout









Figure 140: Junction 47 Traffic Condition

A.1.48 JUNCTION 48: CHURCH STREET / QUEEN STREET

Junction 48 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

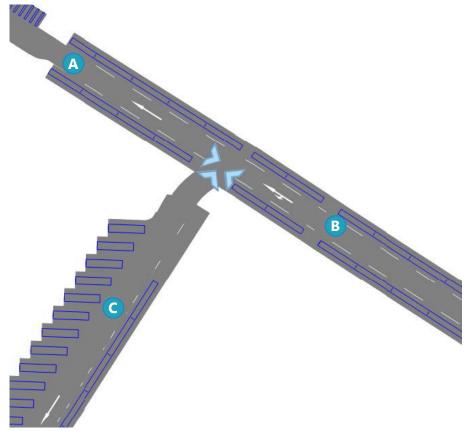


Figure 141: Junction 48 Layout







Figure 142: Junction 48 Traffic Condition

A.1.49 JUNCTION 49: CHURCH STREET / JALAN LEBUH KAPITAN KELING

Junction 49 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Medium-length queues were observed on approaches of the junction.

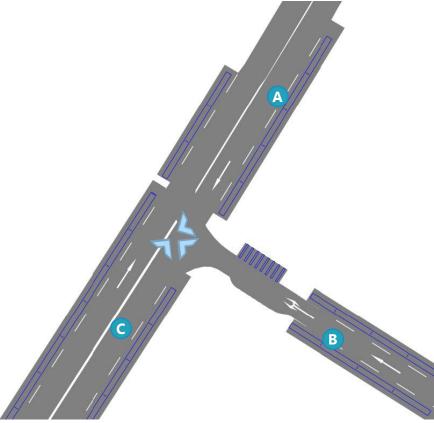


Figure 143: Junction 49 Layout







Figure 144: Junction 49 Traffic Condition

A.1.50 JUNCTION 50: LORONG ARGUS / JALAN LEBUH KAPITAN KELING

Junction 50 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction during the morning and evening

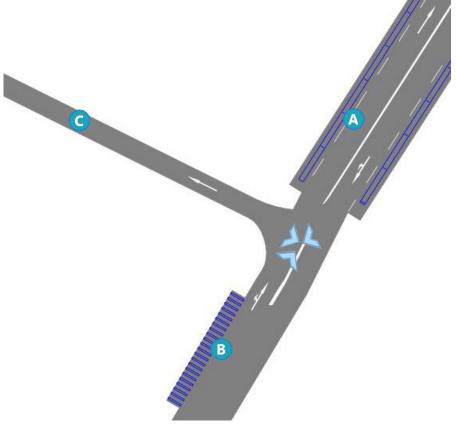


Figure 145: Junction 50 Layout







Figure 146: Junction 50 Traffic Condition

A.1.51 JUNCTION 51: LEBUH BISHOP / JALAN LEBUH KAPITAN KELING

Junction 51 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

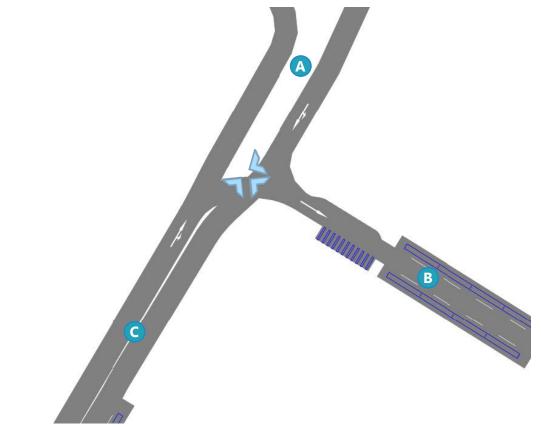


Figure 147: Junction 51 Layout







Figure 148: Junction 51 Traffic Condition

A.1.52 JUNCTION 52: LEBUH BISHOP / LEBUH KING

Junction 52 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

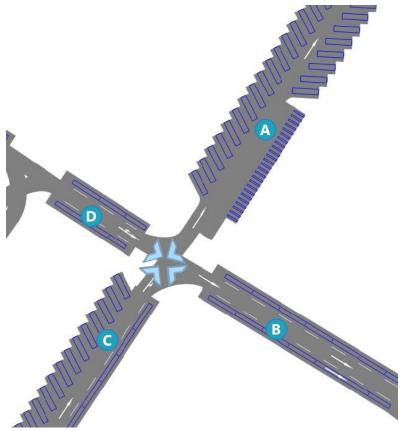


Figure 149: Junction 52 Layout









Figure 150: Junction 52 Traffic Condition

A.1.53 JUNCTION 53: LEBUH BISHOP / PENANG STREET

Junction 53 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

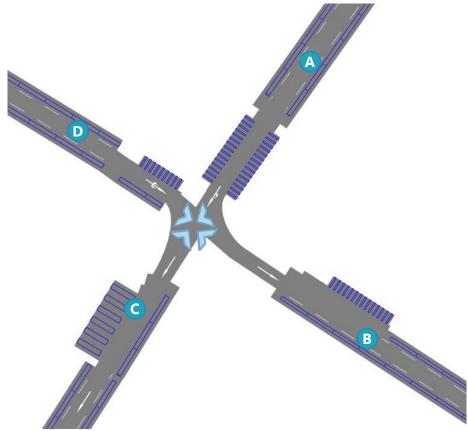


Figure 151: Junction 53 Layout









Figure 152: Junction 53
Traffic Condition

A.1.54 JUNCTION 54: LEBUH UNION / PENANG STREET

Junction 54 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

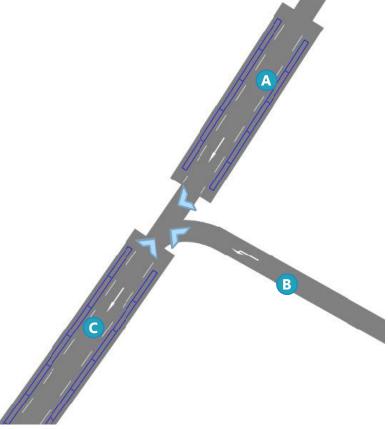


Figure 153: Junction 54 Layout







Figure 154: Junction 54
Traffic Condition

A.1.55 JUNCTION 55 LEBUH LIGHT / PENANG STREET

Junction 55 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

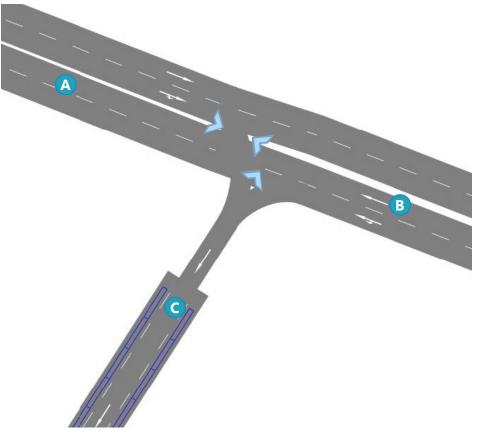


Figure 155: Junction 55 Layout







Figure 156: Junction 55 Traffic Condition

A.1.56 JUNCTION 56 LEBUH LIGHT / LEBUH KING

Junction 56 is a signalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

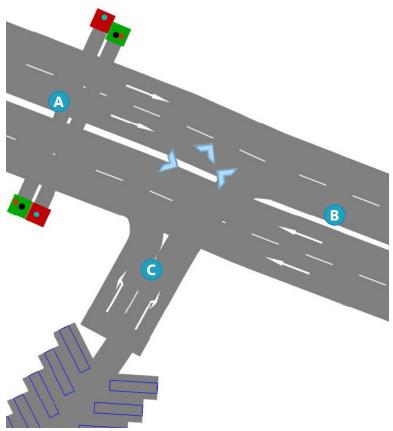


Figure 157: Junction 56 Layout









Figure 158: Junction 56 Traffic Condition

A.1.57 JUNCTION 57 LEBUH LIGHT / JALAN PADANG KOTA LAMA

Junction 57 is a signalized T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

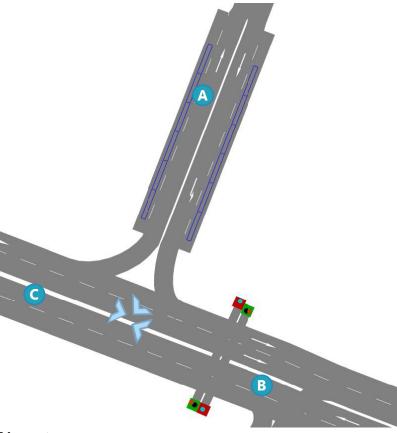


Figure 159: Junction 57 Layout







Figure 160: Junction 57 Traffic Condition

A.1.58 JUNCTION 58 LEBUH LIGHT / JALAN MASJID KAPITAN KELING

Junction 58 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

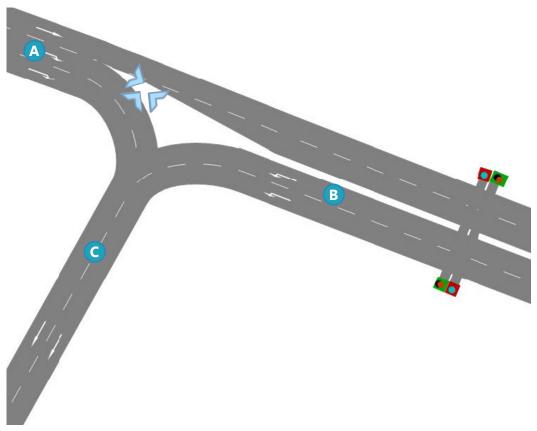


Figure 161: Junction 58 layout







Figure 162: Junction 58 Traffic Condition

A.1.59 JUNCTION 59 LEBUH FARQUHAR / JALAN MASJID KAPITAN KELING

Junction 59 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

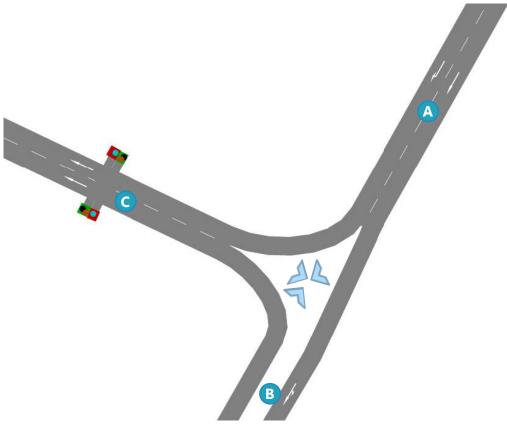


Figure 163: Junction 59 Layout







Figure 164: Junction 59 Traffic Condition

A.1.60 JUNCTION 60 LEBUH LIGHT / JALAN TUN SYED SHEH BARAKBAH

Junction 60 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

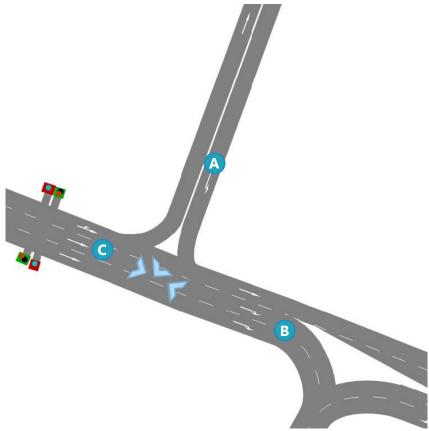


Figure 165: Junction 60 Layout







Figure 166: Junction 60 Traffic Condition

A.1.61 JUNCTION 61 LEBUH FARQUHAR / ACCESS

Junction 61 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

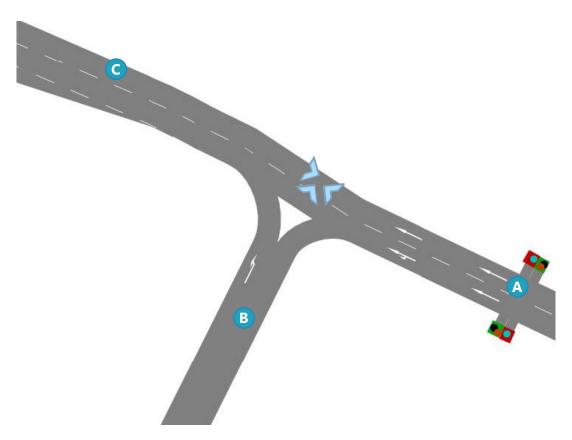


Figure 167: Junction 61 Layout







Figure 168: Junction 61 Traffic Condition

A.1.62 JUNCTION 62 LEBUH LIGHT / JALAN GREEN HALL

Junction 62 is a unsignalised T junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

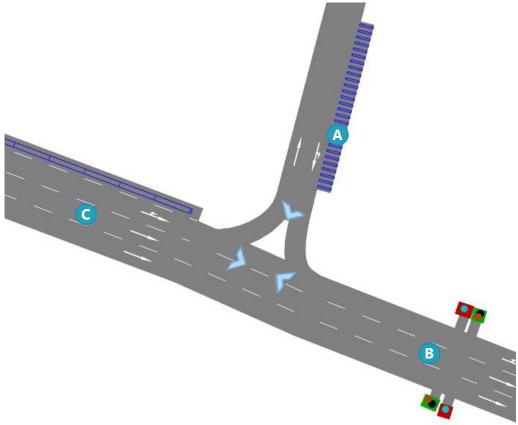


Figure 169: Junction 62 Layout







Figure 170: Junction 62 Traffic Condition

A.1.63 JUNCTION 63 LEBUH FARQUHAR / LEBUH LIGHT

Junction 63 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

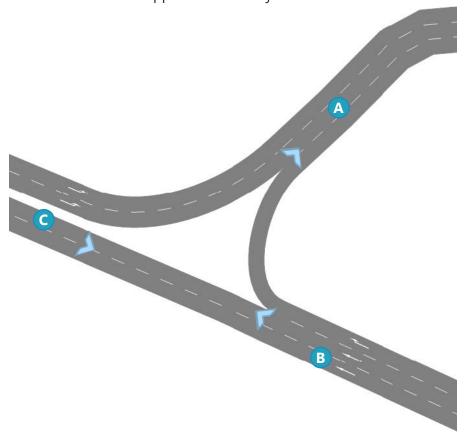


Figure 171: Junction 63 Layout







Figure 172: Junction 63
Traffic Condition

A.1.64 JUNCTION 64 JLN SULTAN AHMAD SHAH / LORONG LOVE

Junction 64 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

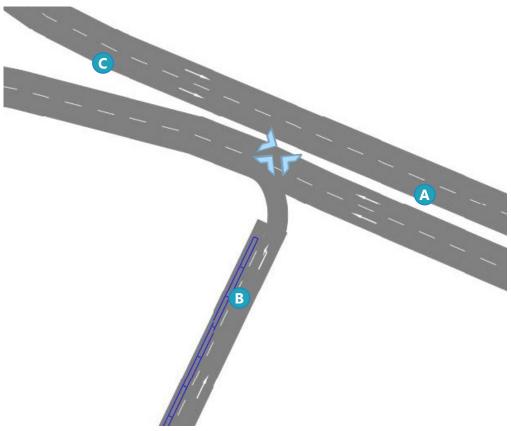


Figure 173: Junction 64 Layout







Figure 174: Junction 64
Traffic Condition

A.1.65 JUNCTION 65 LORONG ARGUS / LORONG LOVE

Junction 65 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction

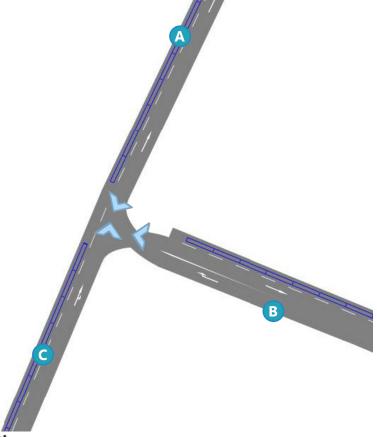


Figure 175: Junction 65 Layout







Figure 176: Junction 65
Traffic Condition

A.1.66 JUNCTION 66 LORONG STEWART / LORONG LOVE

Junction 66 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

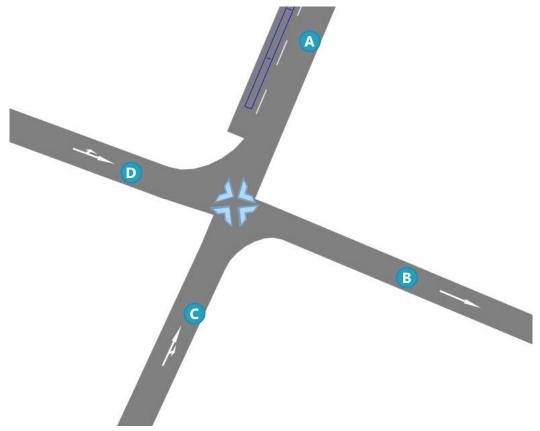


Figure 177: Junction 66 Layout

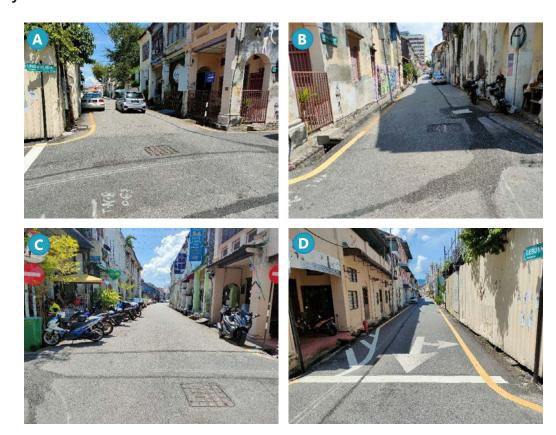


Figure 178: Junction 66 Traffic Condition

A.1.67 JUNCTION 67 LORONG STEWART / LORONG CHULIA

Junction 67 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

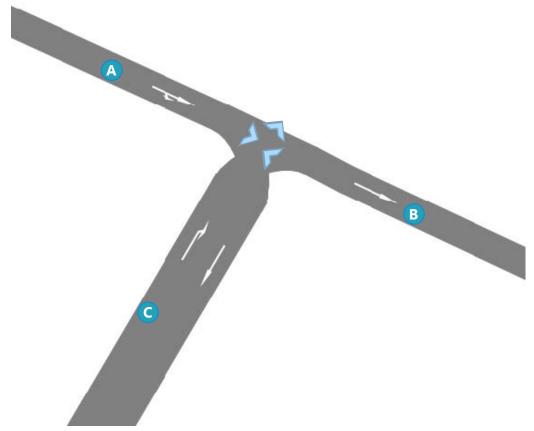


Figure 179: Junction 67 Layout







Figure 180: Junction 67
Traffic Condition

A.1.68 JUNCTION 68 LEBUH CARNAVON / CHULIA STREET

Junction 68 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

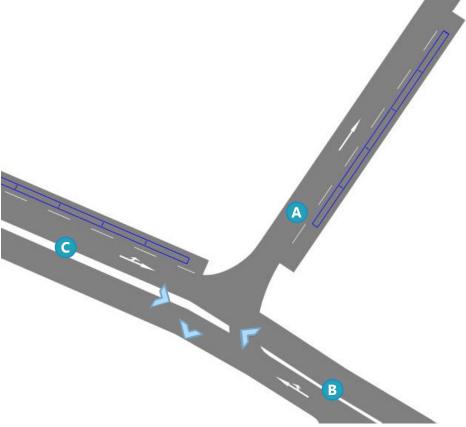


Figure 181: Junction 68 Layout







Figure 182: Junction 68 Traffic Condition

A.1.69 JUNCTION 69 LEBUH CARNAVON / CHULIA STREET

Junction 69 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

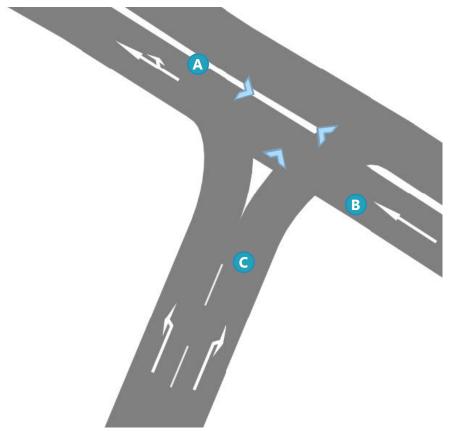


Figure 183: Junction 69 Layout







Figure 184: Junction 69 Traffic Condition

A.1.70 JUNCTION 70 LORONG CHULIA / CHULIA STREET

Junction 70 is a unsignalised T junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

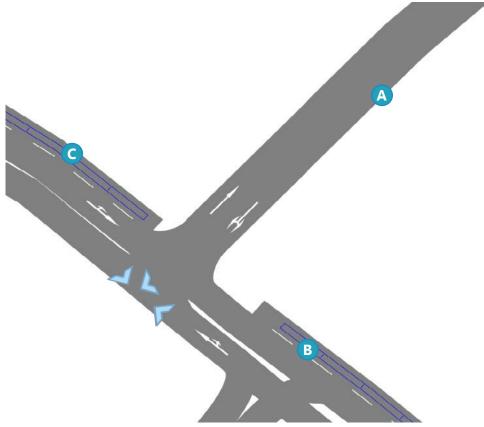


Figure 185: Junction 70 Layout







Figure 186: Junction 70 Traffic Condition

A.1.71 JUNCTION 71 JALAN CAMPBELL / LEBUH CARNAVON

Junction 71 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

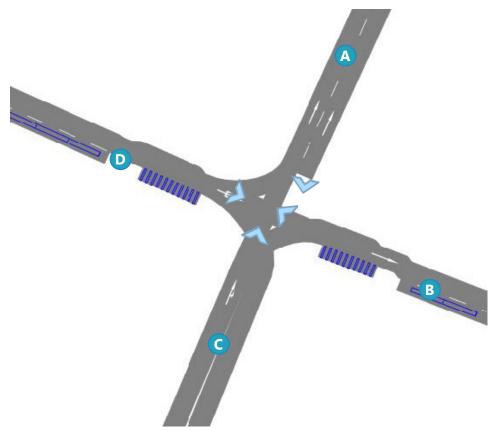


Figure 187: Junction 71 Layout







Figure 188: Junction 71 Traffic Condition

A.1.72 JUNCTION 72 LEBUH CARNAVON / JALAN KAMPUNG KOLAM

Junction 72 is a unsignalised cross junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

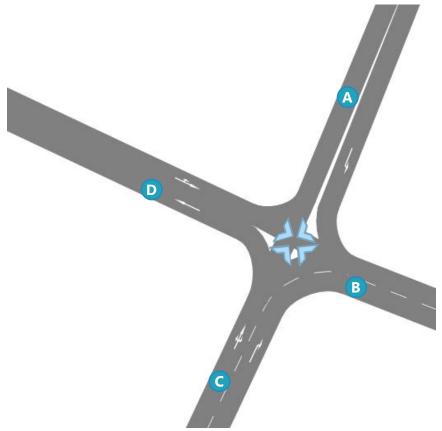


Figure 189: Junction 72 Layout



Figure 190: Junction 72 Traffic Condition

A.1.73 JUNCTION 73 LEBUH CARNAVON / JALAN KAMPUNG KOLAM

Junction 73 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

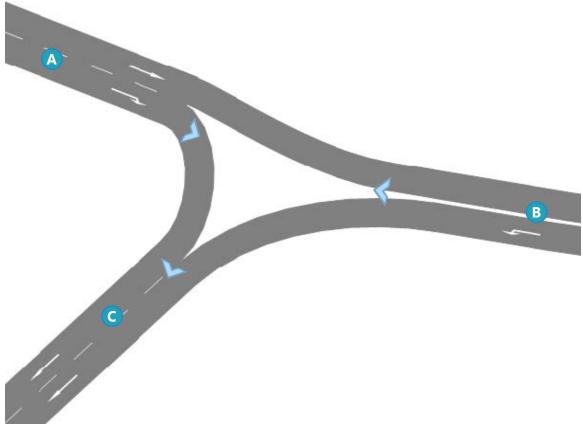


Figure 191: Junction 73 Layout







Figure 192: Junction 73
Traffic Condition

A.1.74 JUNCTION 74 LEBUH CARNAVON / LEBUH ACHEH

Junction 74 is a unsignalised T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

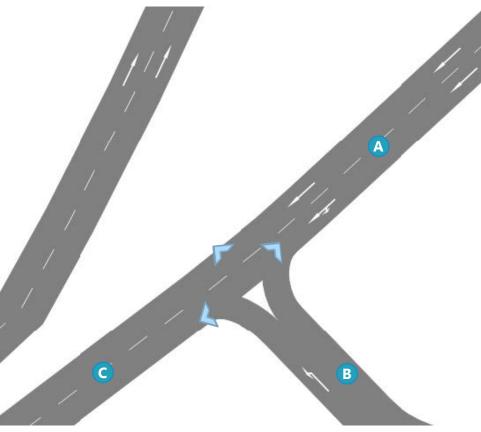


Figure 193: Junction 74 Layout







Figure 194: Junction 74
Traffic Condition

A.1.75 JUNCTION 75 LEBUH CARNAVON / LORONG CARNAVON

Junction 75 is a unsignalized double T-junction and the layout is shown in the figure below, followed by the site photos. Short queues were observed on approaches of the junction.

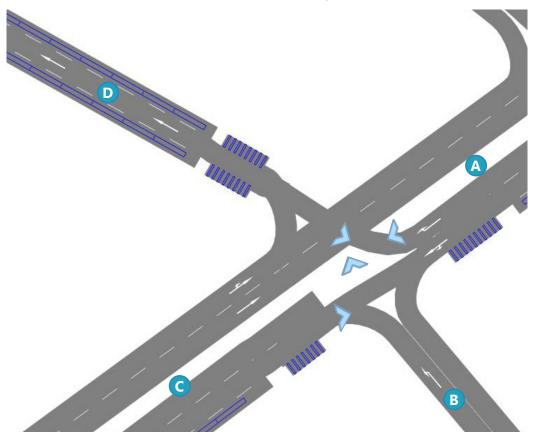


Figure 195: Junction 75 Layout







Figure 196: Junction 75
Traffic Condition

APPENDIX 2: SURVEY HEALTH & SAFETY

Traffic surveys have been conducted by our appointed Traffic Survey subconsultant in Malaysia, Fox Traffic Sdn Bhd. Fox Traffic conducted a health and safety briefing for all survey staff and manage health and safety requirements in accordance with local practices and requirements. The Fox Traffic Health and Safety briefing includes:

- Taking appropriate steps to protect survey staff from the dangers of traffic and/or transport systems and have regard for the rules and regulations that govern the conduct of passengers in the interests of safety.
- Requiring staff to wear the high visibility clothing given to survey staff and make full use of any other equipment designed to improve safety.
- Briefing survey staff to take no action that can, as a consequence, result in others suffering any danger or potential danger. Survey staff should also have regard to safety of others and warn them if survey staff foresee any dangerous consequences of their actions.
- Requiring that survey staff's actions cause no unnecessary obstruction of nuisance.
- Whilst every care is taken in organizing and scheduling survey work to avoid potentially dangerous situations, survey staff should not persevere with any survey assignment if survey staff feel that personal safety is unreasonably at risk.
- Staff involved in manual traffic surveys must obey all instructions given to them by their Manager/ Supervisor. Remembering that politeness and courtesy are essential when dealing with the public, and in that way conflict situation can be avoided.
- Employees experiencing incidents of physical or verbal violence must report this immediately to their Manager/Supervisor, who will in turn report the incident to the appropriate local authority. Any incident of this nature must also be passed on to the Health and Safety manager at the earliest opportunity.



ABOUT THE ASEAN AUSTRALIA SMART CITIES TRUST FUND

The ASEAN Australia Smart Cities Trust Fund (AASCTF) assists ASEAN cities in enhancing their planning systems, service delivery, and financial management by developing and testing appropriate digital urban solutions and systems. By working with cities, AASCTF facilitates their transformation to become more livable, resilient, and inclusive, while in the process identifying scalable best and next practices to be replicated across cities in Asia and the Pacific.





